Rybak Geotechnical, Inc. *Consulting Engineers*

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Updated Geotechnical Engineering Investigation
Proposed Commercial Development
24406 San Fernando Road
Santa Clarita, California
For

AVERY COMPANY
Project No. 1795 November 14, 2007

Rybak Geotechnical, Inc. *Consulting Engineers*

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November 14, 2007
Project No. 1795

Avery Company
22777 Lyons Avenue, Suite 203
Santa Clarita, California 91321

Attn: Gary Avery

Dear Mr. Avery:

We are pleased to submit our report on an updated geotechnical engineering investigation performed for future construction of a commercial development at 24406 San Fernando Road, Santa Clarita, California. Our geotechnical engineering conclusions and recommendations, together with the supporting field and laboratory data, are presented in this report.

Seven copies of the report have been transmitted herewith. It should be noted that no copies of this report have been submitted by us to any other individuals or agencies. Should you have any questions regarding the report or submittals to government agencies, please do not hesitate to call.

Respectfully submitted,
RYBAK GEOTECHNICAL, INC.

RICHARD RYBAK
President

Distribution: (7) Addressee

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INTRODUCTION

This report presents the results of the updated geotechnical engineering investigation performed for the subject property. No definite plans have yet been prepared for construction on the site, however, it is currently anticipated that a commercial/mixed-use development will be constructed in the future. The objectives of the investigation were to evaluate the soil conditions at the site and to provide updated geotechnical engineering recommendations for design and construction of the proposed development.

The scope of our services included subsurface exploration and sampling, laboratory testing, engineering analysis, research of pertinent geologic and geotechnical literature, and preparation of this report. Results of the subsurface exploration and laboratory testing are provided in the Appendix. The scope of our investigation does not include a determination of type/amounts of any potential contaminants in the earth materials. We understand that this determination is being performed by others.

INTENT

It is the intent of this report to aid in the design and completion of the proposed project. Implementation of the "Conclusions and Recommendations" section of this report is intended to reduce certain geotechnical risks associated with construction of the project. The professional opinions and geotechnical advice contained in this report are subject to the general conditions described in the "Limitations" section of this report.

SITE DEVELOPMENT

It is anticipated that a commercial development, possibly containing some mixed-use construction, may be constructed some time in the future. This office had previously prepared a report entitled, *Preliminary Geotechnical Engineering Investigation, Proposed Commercial Development, 24406 San Fernando Road, Santa Clarita, California*, dated July 20, 2006. Since the preparation of that report, the client has requested additional exploration on portions of the subject property not previously accessible.

The proposed development is anticipated to be up to four or five stories above ground, with up to one level of parking below ground. The above information was provided by the client, Avery Company. No plans or topographic surveys of any kind have yet been provided to this firm. Grading is anticipated to include excavating up to 12 feet for the potential subterranean level, removal and recompaction of existing unsuitable site soils, and placement of basement wall backfill.

SITE DESCRIPTION

The subject site consists of one square block surrounded by San Fernando Road on the east side, Lyons Road on the north side, Railroad Avenue on the east side, and 9th Street on the south side. At the time of exploration, the subject site was occupied by three buildings, a trailer office and miscellaneous out-buildings. A large portion of the subject site is concrete or asphalt-paved. No information was provided to this office which would suggest the presence of underground structures such as tanks, basements or vaults. The site is relatively level, with no pronounced highs or lows. The site is practically void of vegetation due to the commercial nature of the property.

EXPLORATION

Subsurface exploration at the subject site was performed on June 28, 2006 and August 25, 2007, and consisted of the excavation of ten borings to depths ranging between 16.5 and 40 feet below existing surface grade. The borings were excavated with the aid of a hollow-stem auger drilling machine, equipped with 8-inch diameter augers.

The earth materials encountered during excavation of the borings were logged by the field engineer and classified by visual examination in accordance with the Unified Soil Classification System. Relatively undisturbed samples of the earth materials were obtained in the borings with the aid of a thin wall sampler. The sampler was driven by impact from a 140 pound weight dropped 30 inches onto the sampling unit. The soil was retained in brass rings of 2.5-inch outside diameter and 1.0-inch height. Bulk samples of the surficial soils were also collected. Standard Penetration Test (SPT) soundings were also performed at varying depth intervals.

Upon completion of sampling, each boring was properly backfilled. The boring locations are indicated on the enclosed Plot Plan and the soils encountered are shown on the logs, Plates A-1 through A-13 in the Appendix.

LABORATORY TESTING

Representative samples of the site soils were transported to the laboratory and tested to determine pertinent geotechnical engineering properties. The results of the laboratory tests performed are summarized in the Appendix.

The dry density and moisture content were determined for the soils collected by the samplers. The dry unit weights and moisture contents are provided on the logs, Plates A-1 through A-13, and on Table 2 in the Appendix.

Four direct shear tests were performed with the purpose of establishing the shear strength of the soils. All samples were artificially saturated prior to testing. The method of testing was in conformance with ASTM D 3080. The shear testing was performed with a strain-controlled device. The samples were subjected to shearing forces under normal stresses of 1.0, 2.0, and 3.0 kips per square foot. The results of the direct shear tests are provided on Plates B-1 through B-4 in the Appendix.

Seven consolidation tests were performed on ring samples of the natural soils to determine the load-settlement characteristics of the soils at their field moisture content and in a saturated condition. The method of testing was in conformance with ASTM D2435. The results of the consolidation tests are provided on Plates C-1 through C-7 in the Appendix.

The relationship between water content and dry unit weight (compaction curve) of the surficial soils was determined by performing two compaction tests on bulk samples in conformance with ASTM D 1557. The results are provided on Table 3 in the Appendix.

Two expansion index tests were performed on bulk samples representing the surficial soils. The tests were performed in accordance with ASTM 4829 to assess soil expansion potential during

changes in moisture content. The results of the expansion index testing are provided on Table 3 in the Appendix.

Testing was also performed on representative samples of the soils to determine corrosion potential. Resistivity, pH, water-soluble sulfate and chloride content were determined in conformance with California Test Methods 532, 643, 417, and 422, respectively. The results of the corrosion tests are provided on Plate D in the Appendix.

SUBSURFACE CONDITIONS

EARTH MATERIALS

In general, the top one to 3.5 feet of earth materials encountered in our borings are suspected to consist of existing fill. Locally, in boring B8, it is suspected that up to 16 feet of existing fill was encountered. It is suspected that this localized deep fill condition may be due to an old underground tank excavation.

The suspected existing fill generally consists of sand and silty sand, which are brown, moist, medium-dense to locally dense, and contain varying amounts of gravel. The earth materials underlying the suspected existing fill consist of natural, alluvial soils which generally consist of sand and silty sand, with minor amounts of sandy silt. The natural alluvial soils are light to medium brown, tan, orange-brown, moist, medium-dense to dense, and contain varying amounts of gravel and cobbles.

GROUNDWATER

Groundwater was not encountered in any of the borings to a total depth of 40 feet. According to the *Seismic Hazard Zone Report for the Newhall 7.5-Minute Quadrangle, Los Angeles County, California*, prepared by the California Geological Survey, dated 1997, the highest historic depth to groundwater in the immediate vicinity of the subject site is greater than 75 feet. Groundwater is not anticipated to be a problem during construction of the proposed development. Fluctuations in the level of groundwater may occur due to variations in rainfall, temperature, and other factors not

evident at the time of the measurements reported herein. Fluctuations also may occur across the site. High groundwater levels can result in changed conditions.

FAULTING AND SEISMICITY CONSIDERATIONS

GENERAL

The subject site is within the southern portion of the Transverse Ranges geomorphic province and north of the Peninsular Ranges geomorphic province. The Transverse Ranges are dominated by east-west trending, reverse and thrust faults. The Peninsular Ranges are dominated by northwest-trending, strike-slip faults.

REGIONAL FAULTING

Based on criteria established by the California Geological Survey (CGS), faults may be categorized as active, potentially active or inactive. Active faults are those which show evidence of surface displacement within the last 11,000 years (Holocene-age). Potentially-active faults are those that show evidence of last displacement within the last 1.6 million years (Quaternary-age). Faults showing no evidence of displacement within the last 1.6 million years may be considered inactive for most purposes, except for some critical structures.

In 1972, the Alquist-Priolo Special Studies Zones Act (now known as the Alquist-Priolo Earthquake Fault Zoning Act) was passed into law. The Act defines "active" and "potentially active" faults utilizing the same aging criteria as that used by the CGS, above. However, the established policy is to zone only those potentially active faults that have a relatively high potential for ground rupture. Therefore, not all faults termed "potentially active" by the CGS are zoned under the Alquist-Priolo Act. The subject site is not located within any of the state's Alquist-Priolo Earthquake Fault Zones. In addition, no known, mapped active or potentially active faults traverse through the site.

The nearest, known, mapped faults to the subject site include the Holser fault, located about 2.9 miles away, the San Gabriel fault, located about 3 miles away, and the Santa Susana fault, located about 3.5 miles away.

Seismic sources other than faults with known surface expression are so-called "buried thrust faults". These faults are not exposed at the surface and are typically broadly defined based on the analysis of seismic wave recordings of several hundreds of small earthquakes in the southern California area.

Two major buried thrust faults in the Los Angeles area are the Elysian Park fold and thrust belt and the Torrance-Wilmington fold and thrust belt. It is postulated that the Elysian Park structure was responsible for the magnitude 5.9, October 1, 1987 Whittier Narrows earthquake. It is postulated that the Torrance-Wilmington structure was responsible for the magnitude 5.0, January 19, 1989 Malibu earthquake. It is believed that the magnitude 6.7, January 17, 1994 Northridge earthquake was caused by a blind section of the Oak Ridge system located beneath the San Fernando Valley.

Due to the buried nature of these thrust faults, their existence is usually not known until they produce an earthquake. The risk for surface rupture potential of these buried thrust faults is inferred to be low. However, the seismic risk of these buried structures in terms of recurrence and maximum potential magnitude is not yet well established. Therefore, the potential for surface rupture on these surface-verging splays at magnitudes higher than 6.0 cannot be totally precluded.

SITE SEISMICITY

Being located in southern California, the site has been subject to ground shaking in the past on numerous occasions. Moderate to strong ground shaking resulting from earthquakes occurring in southern California can be expected within the lifespan of the proposed development. Being located in southern California, the subject site has been subject to ground shaking in the past on numerous occasions. The strongest, most-recent event to impact the site was the January 17, 1994, magnitude 6.7 Northridge earthquake. The fault believed responsible for the Northridge earthquake is the eastern blind portion of the Oak Ridge fault. The epicenter of the Northridge earthquake was located about 11.5 miles to the south, in the Northridge/Reseda area of the San Fernando Valley.

Other recent earthquakes on relatively nearby faults to have impacted the site include the February 9, 1971, magnitude 6.4 San Fernando earthquake and the October 1, 1987, magnitude 5.9 Whittier

Narrows earthquake. The San Fernando earthquake occurred on the San Fernando fault, the epicenter located about 7.6 miles to the south. Unlike the fault responsible for the Northridge earthquake, rupture on the San Fernando fault propagated to the surface in the Sylmar-San Fernando area of the San Fernando Valley. The Whittier earthquake is believed to have occurred on the Elysian Park fold and thrust belt.

Historic earthquake epicenters from 1800 to 2000 within a 60 mile radius of the site and of magnitude five and higher are shown on Table 2 in the Appendix.

GROUND MOTION PARAMETERS

GENERAL

The ground motion anticipated at a site may be investigated in two ways: a deterministic evaluation and a probabilistic evaluation. A deterministic evaluation of ground motion involves an evaluation of potentially damaging earthquake sources and seismic events. The deterministic earthquake event magnitudes for any particular fault should be maximum values that are specific to that seismic source. Maximum earthquakes may be assessed by estimating rupture dimensions of each of the faults of concern.

A probabilistic evaluation considers the probability of exceedance of various levels of ground motion and is performed with consideration of risk contributions from several regional faults.

DETERMINISTIC METHOD

The deterministic approach recognizes the maximum earthquake magnitudes postulated on regional faults. The maximum earthquake is the theoretical maximum event which could occur along a fault, and is derived from formulas which correlate the length of the fault trace to the theoretical maximum magnitude earthquake.

Table A in the Appendix presents maximum earthquake magnitudes for regional faults located within 62 miles of the subject site. Expected peak horizontal ground accelerations generated by such

earthquakes is also provided. Estimated ground motion at the site for each earthquake considered was calculated using the attenuation equations by Campbell & Bozorgnia (1997 revised). Table A provides estimated ground motions with the specification of one standard deviation. By estimating ground motions with application of one standard deviation, certain factors that are not considered in the attenuation relations, such as directivity, travel path, fault geometry, rupture characteristics, etc., are considered implicitly.

Using this methodology, the highest peak horizontal ground acceleration that is expected from a maximum earthquake is generated by a magnitude 6.5 event on the Holser fault, generating an acceleration at the site on the order of 0.9 g.

PROBABILISTIC METHOD

The probabilistic method uses earthquake activity levels, earthquake magnitude distributions, fault lengths, and other parameters which are estimated for each of the regional faults. The probability of exceedance of various levels of ground motion is calculated by summing the risk contributions of all of the regional faults to obtain values for the site.

According to the 1997 Uniform Building Code (UBC), the structural design should take into account the ground motion from an earthquake with ground motion having a 10 percent probability of being exceeded in 50 years (475 year return period).

The peak horizontal ground accelerations anticipated at the site from the 475 year return event were determined from *Seismic Hazard Report Of The Newhall 7.5-Minute Quadrangle, Los Angeles County, California*. The referenced document indicates that the peak horizontal ground acceleration having a 10 percent probability of being exceeded in 50 years anticipated at the subject site is 0.83 g. This ground motion was estimated using attenuation relations that relate earthquake shaking with magnitude, distance from the earthquake, and type of fault rupture. The attenuation relations of Boore and others (1997), Campbell (1997), Sadigh and others (1997), and Youngs and others (1997) were used to calculate the ground motion. Alluvium conditions are assumed. The predominant earthquake magnitude is 6.6.

SECONDARY SEISMIC HAZARDS

FAULT RUPTURE

The results of geologic literature research indicate that the subject site is not underlain by any known, mapped active or potentially active fault deemed capable of rupturing the surface. The site is not located within any Alquist-Priolo Earthquake Fault Zone. The potential for fault rupture on this site is considered very low.

LIQUEFACTION

The subject property is not mapped as being within a potential liquefaction zone per the State of California Seismic Hazard Zone Map, Newhall quadrangle. The potential for liquefaction occurring beneath the subject site is therefore considered to be very low.

LANDSLIDING

Due to the relatively level nature of the site, the potential for seismically-induced landsliding during an earthquake is considered remote.

DYNAMIC SETTLEMENT

Apart from liquefaction-induced settlements, settlement of loose, unsaturated granular soils during an earthquake can be problematic. Due to the relatively uniform nature of the underlying soils, excessive differential settlements due to earthquake shaking are not anticipated.

CONCLUSIONS AND RECOMMENDATIONS

GENERAL

Development of the site is considered feasible from a geotechnical engineering standpoint, provided the following recommendations are implemented during design and construction. The design of the

proposed project is still very preliminary. This office should be kept informed of all design revisions so that the recommendations contained herein can be appropriately revised, if necessary. Prior to going to bid, the geotechnical engineer should review and approve the architectural, civil and structural plans. Additional exploration may be warranted in the future, depending on the final scope of the project.

A meeting between representatives of the contractor, the geotechnical engineer, the architect, and the owner should be held at the job site at the time equipment is at the site and work is about to commence. The purpose of the meeting is to review the responsibilities of each member of the team.

In general, between one and 3.5 feet of the surficial earth materials encountered during our exploration are suspected to consist of existing fill. Existing fill up to 16 feet deep is suspected locally along the north side of the subject property. The earth materials underlying the existing fill consist of Holocene-age, alluvial sediments. In general, the site soils down to the total depth of 40 feet consist of sands and silty sands, with minor amounts of sandy silt. Please be aware that deeper, localized fill deposits may exist on the subject property.

The existing fill materials are not considered satisfactory, in their present state of compaction, to be utilized for support of foundations, flatwork, paving, or additional structural fill. The existing fill would require removal and replacement as properly compacted fill. Based on the type of future construction anticipated, shallow, conventional foundations may be utilized for foundation support of structures. At-grade structure foundations should be supported on a minimum of 5 feet of properly compacted fill. The compacted fill should extend out horizontally from the edge of footings a minimum distance of 3 feet or the depth of fill beneath the footings, whichever is more. Foundations for subterranean structures may be embedded in the competent natural soils encountered at a minimum depth of 6 feet below existing surface grade. New at-grade building footings will require deepening into the competent soils found below a depth of 6 feet in the proximity of the property lines since extending recompaction horizontally away from the footings will not be possible. This will most likely require the use of slot cutting or shoring. Additional, detailed recommendations can be provided once the design of the project is underway.

Our field representative should further evaluate the actual limits and depth for the excavation and recompaction during construction. Deeper excavation and recompaction may be required in local areas if loose or soft soils are encountered.

Floor slabs may be designed and constructed as conventional slabs-on-grade, provided they are supported either on properly compacted fill or the competent natural soils found at a minimum depth of 6 feet below existing surface grade.

Exterior concrete flatwork and paving should be supported on a minimum of 18 inches of properly compacted fill. During construction, it is recommended that a representative of this office observe the bottom of overexcavation to determine if soft/loose conditions call for more than 18 inches of removal and recompaction.

SEISMIC DESIGN CONSIDERATIONS

Although no known active faults traverse through the subject site, like most of Southern California, the subject site lies within a seismically active area. Earthquake resistant structural design is recommended. Designing structures to be earthquake-proof is generally considered to be impractical, especially for private projects, due to cost limitations. Significant damage to structures may be unavoidable during large earthquakes. If structural design is based on the 1997 Uniform Building Code (UBC) static lateral force procedure , the following minimum seismic parameters should be used:

- Seismic Zone Factor, $Z = 0.4$
- Soil Profile Type, S_D
- Seismic Source Type, B
- Near-Source Factor, $N_a = 1.3$
- Near-Source Factor, $N_v = 1.6$

These minimum code values are intended to protect life and may not provide an acceptable level of protection against significant cosmetic damage and serious economic loss. A significantly higher than code lateral design parameter would be necessary to further reduce potential economic loss

during a major seismic event. Structural engineers, however, often regard higher than code values as impractical for use in structural design. The structural engineer and project owner must decide what level of risk is acceptable and to assign appropriate seismic values for use in structural design. The risk of damage to the future structures due to a large earthquake cannot be totally eliminated, and obtaining appropriate insurance as a mitigation measure is strongly recommended.

If the structural design is based on UBC dynamic lateral-force procedures, we recommend that a horizontal ground acceleration of 0.83g, based on the computed peak acceleration for a 50-year exposure and 10% exceedance, be used with the normalized response spectrum for a soil profile type S_D.

EXPANSIVE SOILS

Laboratory expansion index testing results indicate that the site soils have a very low expansion index. Special considerations with respect to expansive soils are not required.

SOIL CORROSION

Resistivity, pH, chloride and sulfate test results obtained from soil samples obtained at the subject site are presented on Plate D in the Appendix. Minimum electrical resistivities were determined to be 4,700 and 5,200 ohm-centimeters, placing the soils in the moderately corrosive category. The soil pH value was determined to be slightly alkaline at 7.6 and 7.9. Both the chloride and sulfate content of the soils were determined to be low. In summary, the soil is classified as moderately corrosive to ferrous metals and not deleterious to concrete. The use of Type II cement in concrete to be in contact with the soils is recommended.

It is recommended that a consulting corrosion engineer be retained in order to determine the most appropriate protection measures for underground utilities on the project site.

SITE GRADING

Site Preparation and Compaction

It is anticipated that site grading will include excavating for the anticipated subterranean level, backfilling behind basement retaining walls, and removal and recompaction of unsuitable site soils for footing, flatwork, paving and additional compacted fill support. All unsuitable deposits should be removed and replaced as properly compacted fill for support of any footings, floor slabs, concrete paving, and concrete flatwork. After excavating as required, the exposed subgrade soils should be carefully inspected by the geotechnical engineer or his representative to verify the removal of all unsuitable deposits. Unsuitable deposits include existing fill soils, natural soils encountered within 6 feet of the existing surface, vegetation, debris, and loose soils. Subsequently, the exposed soils should be scarified to a depth of 6 inches, brought to near optimum moisture content, and compacted to a minimum of 95 percent of maximum dry density obtainable by the ASTM Designation D1557 method of compaction.

No fill should be placed, spread or rolled during unfavorable weather. When the work is interrupted by rain, operations shall not be resumed until the geotechnical engineer indicates that conditions will permit satisfactory results.

A minimum relative fill compaction of 95 percent of maximum dry density obtainable by the ASTM Designation D1557 method of compaction shall be utilized. After compaction of the exposed soils, all required fill should be placed in loose lifts not more than 8 inches in thickness, brought to near optimum moisture content, and properly compacted.

Excavation Characteristics

The exploratory test excavations were excavated with the aid of a truck-mounted, continuous-flight, hollow-stem auger drilling machine. The soils encountered were granular, very friable, and prone to caving. The contractor should anticipate caving of foundation excavations and in temporary excavations during recompaction operations. Steps should be taken to minimize caving. Oversize materials such as cobbles and boulders will make compaction operations difficult.

Shrinkage

Shrinkage is the loss of soil volume caused by compaction of fills to a higher density than before grading. For earthwork volume estimating purposes, an average shrinkage value of 20 percent may be assumed for the surficial soils. This value is an estimate only and excludes losses due to removal of vegetation, debris, or existing underground structures.

Material for Fill

The on-site soils, less any debris or organic matter, may be used in compacted fills. Any required imported fill should be inspected by the geotechnical engineer prior to stockpiling onsite. Imported soils should consist of relatively granular soils with an Expansion Index of less than 20, and be free of vegetation, debris, and deleterious materials. Fill should be free of rocks larger than 6 inches in any dimension.

Utility Trench Backfill

Backfill of all utility trenches within building, flatwork, parking, and drive areas should be properly compacted. Underground utilities should be provided with proper sand bedding and shading to a minimum of 12 inches above the top of pipe. The remainder of the trench should be properly backfilled with the on-site soils or imported sand and mechanically compacted in lifts to the proper minimum relative compaction. Jetting or flooding of backfill materials is not permitted.

Cesspool Abandonment

Any old cesspools encountered on the site during construction should be properly abandoned during grading operations. Cesspools should be completely cleaned out and filled to within 5 feet plus the depth of the future footings with a minimum two-sack slurry mix. The upper portion should be benched into the surrounding natural soils and the fill tested to 95 percent relative compaction. The entire footing footprint plus 5 feet out from the footing should be underlain by a uniform thickness of compacted fill.

Field Observation

The compaction of all required fill should be observed and tested by a representative of this firm. The observation and testing should include the following:

- Observe the exposed subgrade in areas to receive fill and in areas where excavation has resulted in the desired finished subgrade, observe proof rolling, and delineate areas requiring over excavation.
- Perform visual observations to evaluate the suitability of on-site and import soils for fill placement; collect and submit soil samples for required or recommended laboratory testing, where necessary.
- Perform field density testing to determine the percentage of compaction achieved during fill placement. Verify proper moisture content during grading.

The governmental agencies having jurisdiction over the project should be notified prior to commencement of grading so that the necessary grading permits may be obtained and arrangements may be made for the required inspections.

FOUNDATIONS

Design

At-grade, continuous wall footings may be designed for an allowable bearing value of 2,000 pounds per square foot, and should be a minimum of 12 inches in width, 12 inches into the recommended bearing materials, and 12 inches in depth below the lowest adjacent grade. At-grade, isolated column footings may be designed for an allowable bearing value of 2,500 pounds per square foot, and should be a minimum of 24 inches in width, 18 inches into the recommended bearing materials, and 18 inches in depth below the lowest adjacent grade. Increases in allowable bearing value for increases in footing width or depth for footings are not recommended.

Continuous wall footings embedded into competent natural soils located a minimum of 6 feet below existing surface grade may be designed for an allowable bearing value of 3,000 pounds per square foot, and should be a minimum of 12 inches in width, 18 inches into the recommended bearing materials, and 18 inches in depth below the lowest adjacent grade. Isolated column footings embedded into soils located a minimum of 8 feet below existing surface grade may be designed for an allowable bearing value of 3,500 pounds per square foot, and should be a minimum of 24 inches in width, 18 inches into the recommended bearing materials, and 18 inches in depth below the lowest adjacent grade.

A one-third increase in the above allowable bearing values may be utilized for wind or seismic loads. The recommended bearing values are net values, and the weight of the concrete in the footings may be taken as 50 pounds per cubic foot, and the weight of the soil backfill may be neglected when determining the downward loads.

Resistance to Lateral Loading

Resistance to lateral loading may be provided by soil friction and by the passive resistance of the competent natural soils and properly compacted fill. An allowable coefficient of friction of 0.4 may be used between footings and the competent natural soils and properly compacted fill. The allowable passive resistance of the competent natural soils and properly compacted fill against footings may be assumed to be 250 pounds per cubic foot, to a maximum of 2,000 pounds per square foot. The passive resistance of the earth materials and the frictional resistance between the footings and the supporting soils may be combined without reduction in determining the total lateral resistance.

Settlement

A majority of the settlement of the new foundations is expected to occur upon initial loading. The settlement of the new foundations is not expected to exceed 1/2 inch. Differential settlements between adjacent structural elements are not expected to exceed 1/4 inch.

Installation

To provide proper foundation support, new footings should not surcharge any materials other than properly controlled fill or undisturbed, competent natural soils. Proper footing depth may be determined by extending an imaginary 1:1 plane downward from the bottom edges of the new footings. Proper footing depth is achieved when all of the following conditions are met:

- The bottom of footing is embedded the recommended depth below lowest adjacent grade and into the recommended bearing material.
- The imaginary plane is below any unsuitable soils or existing structures and only properly controlled fill or undisturbed, competent natural soils are intersected by the extended plane.
- For footings supported on compacted fill, the lateral extent of compacted fill beyond the limits of the footings should be a minimum of the depth of compacted fill below the footing, or three feet, whichever is more.

All footing excavations should be cleaned of loose soil and debris and inspected and approved by the geotechnical engineer prior to the placement of reinforcing steel, concrete forms, or concrete. The bottoms of footings should be properly wetted prior to pouring concrete to eliminate any shrinkage cracks. The geotechnical engineer should verify that the footing will penetrate into the recommended bearing material.

FLOOR SLABS

Subsequent to the grading as recommended herein, floor slabs should be supported on properly compacted fill. The garage floor slabs may be supported on either properly compacted fill or the natural alluvial soils at the level of the subterranean garage. Any soils disturbed or over excavated in the area of the new slab should be excavated and replaced with properly compacted fill. Based on test results, the underlying surface soils are very low in expansion potential, however, it is recommended to maintain subgrade soil in near optimum moisture condition by periodic watering following excavations.

It is recommended that the floor slabs be a minimum of four inches in thickness. Garage slabs should be a minimum of five inches in thickness. All floor slabs should be reinforced with a minimum of #4 steel bars placed 16 inches on center, each way. In order to decrease the potential for moisture to migrate through the floor slab, it is recommended that a fiber reinforcement additive be added to the concrete mix. This will improve the tensile strength of the concrete and reduce the likelihood of shrinkage cracks from developing. In addition, it is important that any shrinkage cracks that do develop in the floor slab be sealed prior to covering of the slab.

The proposed floor slab should be underlain by a minimum 10-mil vapor barrier. The vapor barrier should be lapped and glued in accordance with the manufacturer's specifications and sealed around plumbing. The vapor barrier should be sandwiched between two, two-inch thick layers of clean sand. All joints should be sealed to prevent moisture intrusion.

Construction activities and exposure to the environment can cause deterioration of prepared subgrades. Therefore, we recommend that our field representative observe the condition of the final subgrade earth materials immediately prior to slab-on-grade construction and, if necessary, perform further field density and moisture content tests to determine the suitability of the final prepared subgrade.

Concrete shrinks as it cures, resulting in shrinkage tension within the concrete mass. The development of tension results in cracks within the concrete since concrete is weak in tension. Therefore, the concrete should be placed using procedures to minimize the cracking within the slab. Shrinkage cracks can become excessive if water is added to the concrete above the allowable limit and proper finishing and curing practices are not followed. Concrete mixing, placement, finishing and curing should be performed per the American Concrete Institute Guide for Concrete Floor and Slab Construction (ACI 301.1R-89). Where shrinkage cracks would be unsightly, such as in the garage, concrete slabs on grade should be provided with tooled, crack control joints at 10 to 15-foot centers or as specified by the structural engineer.

Tile flooring can crack, reflecting cracks in the concrete slab below the tile. Therefore, the slab designer should consider additional steel reinforcement in concrete slabs on-grade that will directly support tile. The tile installer should consider installation methods that reduce possible cracking of

the tile. A vinyl crack isolation membrane (approved by the Tile Council of America/Ceramic Tile Institute) is recommended between tile and concrete slabs-on-grade per the Portland Cement Association Specifications.

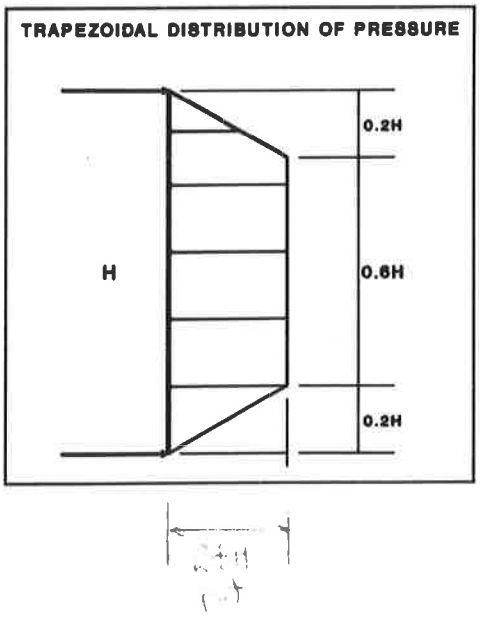
EXTERIOR CONCRETE FLATWORK

Removal and recompaction beneath exterior concrete flatwork was discussed in the "General" subsection of the "Conclusions and Recommendations" section of this report. It is recommended that all exterior concrete flatwork, such as walkways, patios, and other miscellaneous areas be a minimum of 3-1/2 inches in thickness, and be reinforced with 6x6-6/6 welded wire mesh. Driveway pavement should be a minimum of 5 inches in thickness and reinforced with a minimum of #4 bars at 16 inch centers. Any soils disturbed or overexcavated in the area of the new flatwork should be excavated and replaced with fill compacted to at least 95 percent relative compaction.

RETAINING WALLS

Design

Free-standing, non-restrained, cantilever retaining walls may be designed to resist a triangular pressure distribution of lateral earth pressure of 30 pounds per cubic foot. Garage retaining walls up to 12 feet in vertical height, restrained from movement at the top, may be designed to resist a trapezoidal pressure distribution of lateral earth pressure of 24H pounds per square foot, where H is the height of the wall in feet, as indicated in the following diagram.



Additional active pressure should be added for a surcharge condition due to sloping ground, vehicular traffic, or adjacent structures. In addition to the recommended earth pressure, the upper ten feet of the basement walls adjacent to streets should be designed to resist a uniform lateral pressure of 100 pounds per square foot, acting as a result of an assumed 300 pounds per square foot surcharge behind the walls due to normal street traffic. If the traffic is kept back at least ten feet from the basement walls, the traffic surcharge may be neglected.

Wall Drainage

All retaining walls should be provided with a subdrainage system to prevent buildup of storm water, irrigation water, and nuisance or utility water against the back of wall. Typical back-of-wall drainage systems for basements consist of subdrain pipes surrounded by gravel or geotextile drainage composites draped on shored excavations or back of cast walls.

Should pipe and gravel be used, the subdrain pipes should consist of four-inch diameter, perforated pipe embedded in a minimum of one cubic foot per linear foot of gravel wrapped in filter fabric

(Mirafi 140N or equivalent). The perforations should point downwards. The subdrains should be placed at the base of the wall and be sloped to drain to appropriate receptacles by gravity.

In lieu of pipe and gravel, a geocomposite drainage blanket (Miradrain 6000 or equivalent) may be utilized against the back of wall. Geocomposites typically consist of waffle-like, plastic drain material covered by filter fabric. Miradrain strips may be placed at a depth starting at about 2 feet below the finish grade. It is recommended that strips at least 4 feet in width be placed at 8-foot centers. The strips should be connected to a continuous 4-foot wide Miradrain strip placed at the bottom of the excavation. The Miradrain should be connected to a solid drainage pipe. As an alternative to the 4-foot wide continuous strip at the bottom of the excavation, a 4-inch diameter perforated pipe may be used. The pipe should be placed with the perforations down and surrounded by at least one cubic foot of gravel wrapped in filter fabric.

The installed drainage system should be observed by personnel from this firm prior to being backfilled. Inspection of the drainage system may also be required by the reviewing governmental agencies.

Moisture seepage or discoloration of the face of retaining walls due to mineral deposits (efflorescence) is common with retaining walls not provided with proper waterproofing. Depending on the level of moisture protection desired, damp-proofing or waterproofing should be considered.

Wall Backfill

Wall backfill should be mechanically compacted in loose layers not more than 8 inches in thickness, to the proper relative compaction. Flooding or jetting of the backfill is not permitted. Proper compaction of the backfill will be necessary to reduce settlements of the backfill and overlying flatwork.

ASPHALT PAVEMENT DESIGN

It is recommended that no less than 18 inches of properly compacted fill underlie paving sections. In areas where traffic is anticipated to consist of only automobiles and light trucks, a paving section

consisting of 3 inches of A.C. over 5 inches of crushed aggregate base can be utilized. In areas where heavier traffic, such as trash truck, delivery truck, etc., is anticipated, a paving section consisting of 4 inches of A.C. over 6 inches of crushed aggregate base should be utilized. Crushed aggregate base should be compacted to a minimum 95 percent relative compaction.

SITE DRAINAGE

Positive surface gradients should be provided adjacent to all structures so as to direct surface water run-off and roof drainage away from foundations and slabs toward suitable discharge facilities. Drainage should not be allowed to pond anywhere on the site. All structures should be provided with roof gutters and down spouts.

TEMPORARY EXCAVATIONS

General

It is anticipated that temporary excavations up to 14 feet in height may be required during the construction of the subterranean portions of the project. Should the perimeter garage walls be positioned far enough from the property lines, the temporary excavations may be sloped. A uniform 1:1 (horizontal to vertical) gradient should not be exceeded. Due to the cohesion-less nature of the site soils, vertical excavations are not recommended. Where sloped excavations are not possible, all or portions of the excavations will require shoring. Shoring may consist of drilled soldier piles with wood lagging.

Excavation of the shoring piles is anticipated to be difficult due to the presence of oversize rocks, and caving sands, gravels, cobbles, and boulders.

Where sloped embankments are utilized, the tops of the slopes should be barricaded to prevent vehicles and storage loads within 5 feet of the tops of the slopes. If the temporary construction embankments are to be maintained during the rainy season, berms are suggested along the tops of the slopes where necessary to prevent runoff water from entering the excavation and eroding the

slope faces. The soils exposed in the cut slopes can be made if variations in the soil conditions occur. All excavations should be stabilized within 30 days of initial excavation.

Shoring

The following information on the design and installation of the shoring is as complete as possible at this time. It is suggested that a review of the final shoring plans and specifications be made by this office prior to bidding or negotiating with a shoring contractor.

One method of shoring would consist of steel soldier piles, placed in drilled holes and backfilled with concrete. The soldier piles may be designed as cantilevers. Drilled, cast-in-place soldier piles should be placed no closer than 2 diameters on center. The minimum diameter of the piles is 18 inches. Structural concrete should be used for the soldier piles below the excavation; lean-mix concrete may be employed above that level. As an alternative, lean-mix concrete may be used throughout the pile where the reinforcing consists of a wide flange section. The slurry must be of sufficient strength to impart the lateral bearing pressure developed by the wide flange section to the soil. For design purposes, an allowable passive value for the soils below the bottom plane of excavation, may be assumed to be 500 pounds per square foot per foot of depth. To develop the full lateral value, provisions should be implemented to assure firm contact between the soldier piles and the undisturbed soils.

Casing may be required should caving be experienced in the granular soils. If casing is used, extreme care should be employed so that the pile is not pulled apart as the casing is withdrawn. At no time should the distance between the surface of the concrete and the bottom of the casing be less than 5 feet.

It is recommended that the exposed soils be observed by the soils engineer to verify the cohesive nature of the soils and the areas where lagging may be omitted. Due to the granular nature of the existing fill and natural soils, it is anticipated that shored excavations will require complete lagging. Soldier piles should be designed for the full anticipated pressures. Due to arching in the soils, the pressure on the lagging will be less. It is recommended that the lagging be designed for the full design pressure but be limited to a maximum of 400 pounds per square foot.

Cantilever soldier piles retaining up to 14 feet with a level back slope should be designed for a minimum equivalent fluid pressure of 30 pounds per cubic foot. Additional pressure should be added for sloped conditions, adjacent traffic, and structural surcharge conditions. The design movement of the top of the piles should be limited to ½ inch.

Because of the depth of the excavation, some means of monitoring the performance of the shoring system is recommended. The monitoring should include ground surface settlement and lateral deformation. Some movement of the shored embankments should be anticipated as a result of the relatively deep excavation. It is recommended that photographs of the existing improvements on the adjacent properties be made during construction to record any movements for use in the event of a dispute.

PLAN REVIEW

Formal plans ready for submittal to the Department of Building and Safety must be reviewed by Rybak Geotechnical, Inc. prior to submittal. Any change in the scope of the project may require additional geotechnical work.

SITE OBSERVATION

It is recommended that all foundation excavations be observed by the geotechnical engineer prior to placing forms, steel, or concrete. All excavations should be cleaned of loose soil and debris prior to inspection. The excavations should be approved by this office prior to the placement of reinforcing steel, concrete forms, or concrete. The geotechnical engineer should verify that the footings will be embedded into the recommended bearing material. All bottom excavations to receive compacted fill should be inspected by the geotechnical engineer prior to placing any controlled compacted fill. Any fill that is placed must be approved, tested, and verified if used for engineered purposes. Should the observations reveal any unforeseen hazard, additional recommendations may be necessary.

Please advise Rybak Geotechnical, Inc. at least 24 hours prior to any required site visit. All approved plans, permits, and geotechnical reports must be at the jobsite and available during inspections.

CONSTRUCTION SITE MAINTENANCE

The construction contractor or general contractor shall supervise and direct the work and they shall be solely responsible for all construction means, methods, techniques, sequences and procedures. Also, they shall be solely and completely responsible for conditions on the job site, including safety of all persons and property during the performance of work. Periodic or continuous observations by the geotechnical consultant will not, nor are intended to, include review of the adequacy of the contractor's safety measures in, on or near the construction site.

It is the responsibility of the contractor to maintain a safe construction site. When excavations exist on a site, the area should be fenced and warning signs posted. Earth material generated by foundation and subgrade excavations should be either removed from the site or properly placed as a controlled compacted fill. Earth material must not be spilled over any descending slope.

Workers should not be allowed to enter any unshored trench excavations over 5 feet deep. All shoring, bracing and excavation or confined space entry should be in accordance with current requirements of CAL/OSHA, the Industrial Accident Commission of the State of California, and all other public agencies having jurisdiction.

Please call this office with any questions. This report and the exploration are subject to the following "Limitations" section. Please read the "Limitations" carefully, as it limits our liability.

LIMITATIONS

This report was prepared in accordance with generally accepted geotechnical engineering principles and practice available at this time and with the degree of care and skill ordinarily exercised under similar circumstances by geotechnical engineers practicing in this area. No other representation, either expressed or implied, is made as to the conclusions and professional advice included in this report.

This report was prepared exclusively for the sole use and benefit of the client, Avery Company, and their authorized agents, and is not transferrable. This report is intended for use with regard to the

specific project discussed herein. The conclusions and recommendations contained in this report are based on the data relating only to the project and location discussed herein. Any changes in design or locations of structures from those outlined in this report should be provided to us so that we may review our conclusions and recommendations and make any necessary modifications.

The subsurface conditions described in this report have been projected from test pits and/or borings on the site as indicated, and are believed representative of the project area. The conclusions and recommendations contained herein are based on the findings and observations made at the test trench and/or boring locations. It is not unusual to find conditions beyond test trenches and/or borings which differ from the actual conditions encountered. Therefore, existing fill, soil, geologic and groundwater conditions can vary significantly between exploratory excavations and with passage of time. As with most projects, conditions revealed during construction may be at variance with the findings herein. If this occurs, the changed conditions must be evaluated by the geotechnical consultant, and additional recommendations provided, as warranted. The exploration was performed only on a portion of the site, and cannot be considered as indicative of the portions of the site not explored.

The recommendations contained within this report were developed with the assumption that the necessary geotechnical observations and testing will be performed during construction by a representative of this firm. If construction phase services are performed by others, they must accept full responsibility for all geotechnical aspects of the project, including this report.

Respectfully submitted,
RYBAK GEOTECHNICAL, INC.

REGISTERED PROFESSIONAL ENGINEER
RICHARD RYBAK
No. 2131
Exp. 3/31/09
GEOTECHNICAL
STATE OF CALIFORNIA

RICHARD RYBAK
Geotechnical Engineer
G.E. 2131

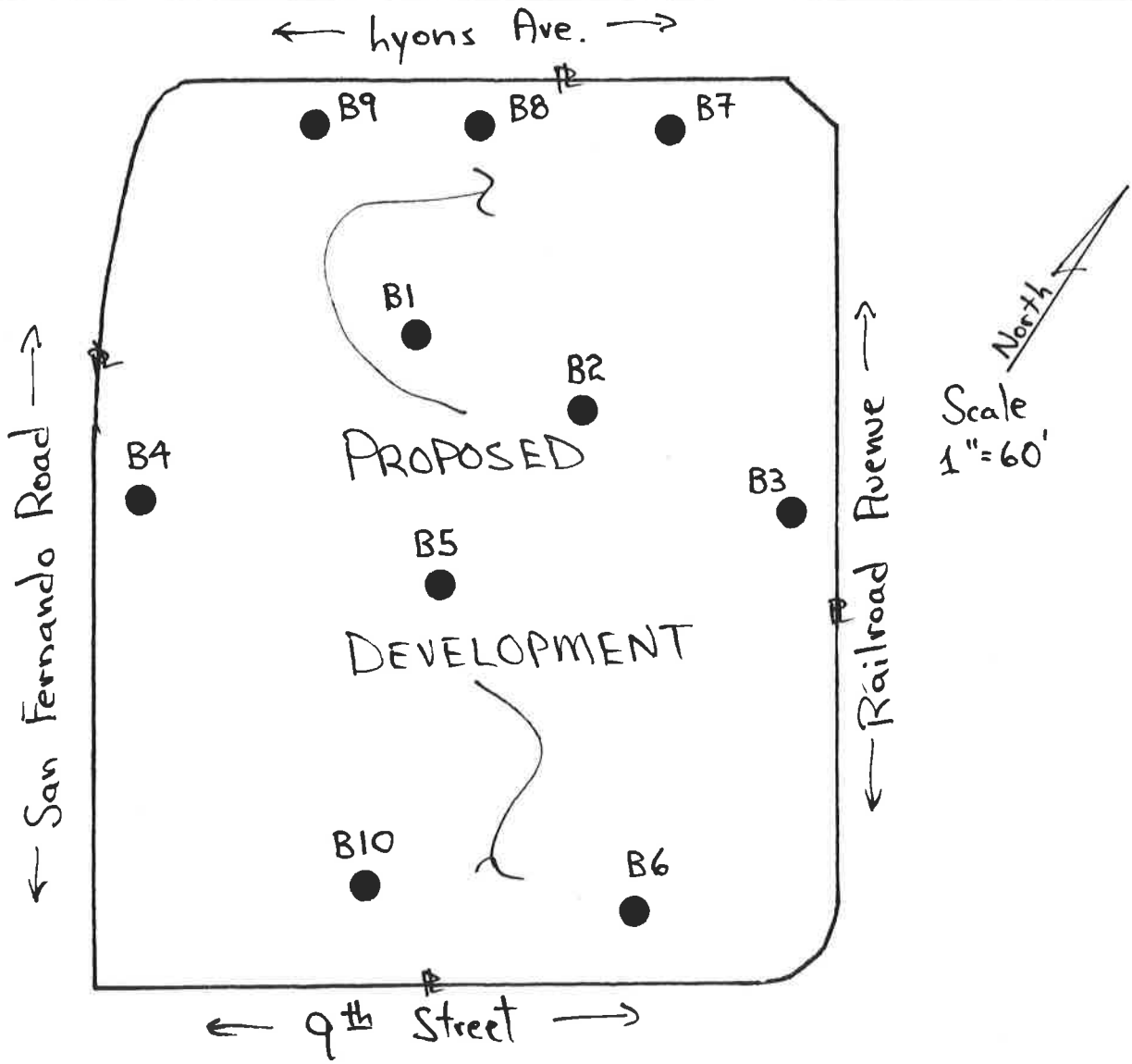
RR:lc

Distribution: (7) Addressee

Rybak Geotechnical, Inc.
Consulting Engineers

BY: RR DATE: 11/2007 CLIENT: AVERY

SUBJECT: PLOT PLAN PROJECT #: 1795



RYBAK GEOTECHNICAL, INC.

BORING NO. 1

Client: AVERY COMPANY
Project No. 1795

Exploration Date: June 28, 2006

Sample Depth, (ft.)	Blows per foot	Moisture Content (%)	Dry Density (lbs/cu.ft.)	Depth (ft.)	Soil Type	Description Surface Conditions:Parking Lot
				0		
				1		SUSPECTED FILL: Silty Sand, brown, moist, medium dense, trace gravel
				2		
2.5	19	4.7	108.5	3	SP	NATURAL SOIL: Gravelly Sand, tan and orange-brown, moist, medium-dense to dense, medium to coarse grained
				4		
5	18	2.9	103.9	5		
				6		
				7		
				8		
				9	SM	Silty Sand, brown, moist, medium dense, trace gravel
10	6	1.9	108.7	10		
				11		
				12		
				13		
				14		
15	18	11	101.0	15	SP	Sand, golden brown, mottled black, moist, medium dense, well graded some gravel
				16		
				17		
				18		
				19		
20	10	12.8	107.6	20	ML	Sandy Silt, brown, moist, firm
				21		
				22		
				23		
				24	SP	Gravelly Sand, brown, orange-brown, moist, fine to coarse-grained
25	38	1.8	--	25		
				26		
				27		
				28		
				29		
30	38	3	108.6	30		

RYBAK GEOTECHNICAL, INC.

BORING NO. 1 (Continued)

Client: AVERY COMPANY
Project No. 1795

Exploration Date: June 28, 2006

Sample Depth, (ft.)	Blows per foot	Moisture Content (%)	Dry Density (lbs/cu.ft.)	Depth (ft.)	Soil Type	Description Surface Conditions:Parking Lot
				30		
				31		
				32		Total Depth: 31.5 feet No Ground Water Suspected Fill To: 2 feet
				33		
				34		
				35		
				36		
				37		
				38		
				39		
				40		
				41		
				42		
				43		
				44		
				45		
				46		
				47		
				48		
				49		
				50		
				51		
				52		
				53		
				54		
				55		
				56		
				57		
				58		
				59		
				60		

RYBAK GEOTECHNICAL, INC.

BORING NO. 2

Client: AVERY COMPANY
Project No. 1795

Exploration Date: June 28, 2006

Sample Depth, (ft.)	Blows per foot	Moisture Content (%)	Dry Density (lbs/cu.ft.)	Depth (ft.)	Soil Type	Description Surface Conditions:Parking Lot
				0		
				1		SUSPECTED FILL: Silty Sand, brown, moist, medium dense, trace gravel
				2	SP	NATURAL SOIL: Sand, moist, brown, medium to medium coarse-grained, some gravel, friable
2.5	20	3.0	116.8	3		
				4		abundant gravel
5	13	2.8	--	5		
				6		
				7		
				8	SP-SM	Sand with silt, golden brown, moist, dense, fine to medium grained, gravel to 1/4"
				9		
10	18	3.4	107.2	10		
				11		
				12		
				13	SW	Sand, light brown, mottled white, moist, dense, fine to coarse grained, gravelly
				14		
15	26	1.9	--	15		
				16		
				17		
				18		Total Depth: 16.5 feet No Ground Water Suspected Fill To: One Foot
				19		
				20		
				21		
				22		
				23		
				24		
				25		
				26		
				27		
				28		
				29		
				30		

RYBAK GEOTECHNICAL, INC.

BORING NO. 3

Client: AVERY COMPANY
Project No. 1795

Exploration Date: June 28, 2006

Sample Depth, (ft.)	Blows per foot	Moisture Content (%)	Dry Density (lbs/cu.ft.)	Depth (ft.)	Soil Type	Description Surface Conditions: Dirt Lot
				0		
				1		SUSPECTED FILL: Sand, brown, moist, medium dense
				2	SP	NATURAL SOILS: Sand, golden brown, moist, some gravel to 1/4"
2.5	18	3.3	107.5	3		
				4		
5	14	2.7	104.0	5		
				6		
				7		
				8		
				9		abundant gravel
10	15	5.0	103.1	10		
				11		
				12		
				13		
				14		
15	19	5.5	110.7	15		
				16		
				17		
				18		
				19		
20	24	3.5	105.3	20		
				21		
				22		
				23		Total Depth: 21.5 No Groundwater Susptected Fill To: 1.5 Feet
				24		
				25		
				26		
				27		
				28		
				29		
				30		

RYBAK GEOTECHNICAL, INC.

BORING NO. 4

Client: AVERY COMPANY
Project No. 1795

Exploration Date: June 28, 2006

Sample Depth, (ft.)	Blows per foot	Moisture Content (%)	Dry Density (lbs/cu.ft.)	Depth (ft.)	Soil Type	Description Surface Conditions: Parking Lot 3" Asphalt, No Base
				0		
				1		SUSPECTED FILL: Silty Sand, brown, moist, medium dense, trace gravel
				2	SP	NATURAL SOILS: Sand, moist, friable, medium grained
2.5	4	2.2	104.5	3		
				4		
5	4	8.4	93.9	5	SM	Silty Sand, brown, moist, medium dense, fine grained, trace gravel
				6		
				7		
				8		
				9		
10	8	8.5	102.2	10		
				11		
				12		
				13		
				14		
15	12	8.9	112.1	15		
				16		
				17		
				18		dense
				19		
20	16	1.8	--	20		
				21	SW	Sand, golden brown-brown, moist, dense, fine to medium grained
				22		
				23		
				24	SP-SM	Sand with silt, brown-golden brown, moist, dense, poorly graded
25	29	1.4	--	25		
				26		
				27		
				28		Total Depth: 26.5' No Ground Water Suspected Fill To: 1.0'
				29		
				30		

RYBAK GEOTECHNICAL, INC.

BORING NO. 5

Client: AVERY COMPANY
Project No. 1795

Exploration Date: June 28, 2006

Sample Depth, (ft.)	Blows per foot	Moisture Content (%)	Dry Density (lbs/cu.ft.)	Depth (ft.)	Soil Type	Description Surface Conditions: Parking Lot
				0		
				1		SUSPECTED FILL: Silty Sand, brown, moist, medium dense
				2	SP	NATURAL SOILS: Sand, golden brown, moist, medium dense to dense, fine to coarse grained
2.5	26	3.7	117.2	3		
				4		
5	13	4.3	107.2	5		
				6		
				7		
				8		
				9	SM	Silty Sand, brown, moist, dense, fine-grained, trace gravel
10	10	10.1	107.1	10		
				11		
				12		
				13		
				14		
15	32	7.2	116.7	15		
				16	SP-SM	Sand with silt, dark golden brown, moist, dense, fine to medium grained
				17		
				18		
				19	SM	Silty Sand, brown, moist, dense, fine grained, trace gravel to 1/16"
20	6	6.9	SPT	20		
				21		
				22		
				23		
				24		
25	15	7.9	SPT	25		
				26		
				27		
				28		Total Depth: 26.5' No Ground Water Susptected Fill To: 1 Foot
				29		
				30		

RYBAK GEOTECHNICAL, INC.

BORING NO. 6

Client: AVERY COMPANY
Project No. 1795

Exploration Date: June 28, 2006

Sample Depth, (ft.)	Blows per foot	Moisture Content (%)	Dry Density (lbs/cu.ft.)	Depth (ft.)	Soil Type	Description Surface Conditions: Driveway Asphalt 1"
				0		
				1		SUSPECTED FILL: Silty Sand, brown, moist, dense
				2	SP	NATURAL SOILS: Sand, brown, moist, dense
2.5	27	2.5	117.6	3		
				4	SM	Silty Sand, brown, moist, medium-dense to dense
5	16	3.3	105.5	5		
				6	SP	Sand, gold-brown, moist, dense, medium to coarse-grained
				7		
				8		
				9		
10	7	13.3	102.2	10		
				11	SM	Silty Sand, brown, moist, dense, fine grained, trace gravel
				12		
				13		
				14	SW	Sand, brown-white, moist, dense, well-graded
15	26	3.5	--	15		
				16		
				17		
				18		
				19	SW	Silty Sand, brown, moist, medium dense, fine grained, trace gravel
20	12	4.6	111.8	20		
				21	SP	Sand, golden brown, moist, dense, well graded, fine to coarse grained
				22		
				23		
				24		
25	38	2.6	105.6	25		
				26		
				27		
				28		
				29		
30	45	2.0	113.4	30		

RYBAK GEOTECHNICAL, INC.

BORING NO. 6 (Continued)

Client: AVERY COMPANY
Project No. 1795

Exploration Date: June 28, 2006

Sample Depth, (ft.)	Blows per foot	Moisture Content (%)	Dry Density (lbs/cu.ft.)	Depth (ft.)	Soil Type	Description Surface Conditions:Parking Lot
				30		
				31		
				32		Total Depth: 31.5" No Ground Water Susptected Fill To: 1 foot
				33		
				34		
				35		
				36		
				37		
				38		
				39		
				40		
				41		
				42		
				43		
				44		
				45		
				46		
				47		
				48		
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				50		
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				57		
				58		
				59		
				60		

RYBAK GEOTECHNICAL, INC.

BORING NO. 7

Client: AVERY COMPANY
Project No. 1795

Exploration Date: August 25, 2007

Sample Depth, (ft.)	Blows per foot	Moisture Content (%)	Dry Density (lbs/cu.ft.)	Depth (ft.)	Soil Type	Description Surface Conditions: Parking Lot - 3 inch thick A.C.
				0		SUSPECTED FILL: Silty Sand, brown, moist, medium dense, trace gravel
				1		
				2	SP	NATURAL SOILS: Gravelly Sand, light to dark tan, moist, dense, medium to coarse-grained, some small gravel
				3		
				4		
5	51	9.8	105	5		
				6		
				7		abundant gravel
				8		
				9		
10	50 for 6"	4.2	--	10		
				11		
				12	SP-SM	Sand with silt, orange-brown, moist, dense
				13		
				14		
15	50 for 5"	4.2	116.3	15		
				16		
				17		
				18		
				19	SW	Gravelly Sand, brown, slighlty moist, dense, abundant gravel up to 1"
20	100 for 6"	4.1	--	20		
				21		
				22		Total Depth: 21.5' No Groundwater Fill Suspected to 2 feet
				23		
				24		
				25		
				26		
				27		
				28		
				29		
				30		

RYBAK GEOTECHNICAL, INC.

BORING NO. 8

Client: AVERY COMPANY

Exploration Date: August 25, 2007

Project No. 1795

Sample Depth, (ft.)	Blows per foot	Moisture Content (%)	Dry Density (lbs/cu.ft.)	Depth (ft.)	Soil Type	Description Surface Conditions: Parking Lot - 3 inch thick A.C.
				0		SUSPECTED FILL: Silty Sand, brown, moist, medium dense, trace gravel
				1		dark brown, mottled dark golden tan, medium-dense
				2		
				3		
				4		
5	20	8.3	120.7	5		
				6		Gravelly sand, brown, moist, friable, abundant cobbles and gravel
				7		
				8		
				9		
10	56	7.6	112.5	10		Sand with Silt, dark yellow brown, moist, dense, some gravel up to 1/2"
				11		
				12		
				13		
				14		
15	56	13.4	109.9	15		gray to black, hydrocarbon odor
				16	SM	Silty Sand, brown, moist, dense, some gravel up to 1/2"
				17		
				18		
				19		
20	50 for 6"	7.6	110.9	20		
				21		
				22		
				23	SP	Gravelly sand, brown, moist, friable, abundant cobbles and gravel
				24		
-	-			25		
				26		
				27		
				28		
				29		
				30		

RYBAK GEOTECHNICAL, INC.

BORING NO. 8

Client: AVERY COMPANY
Project No. 1795

Exploration Date: August 25, 2007

Sample Depth. (ft.)	Blows per foot	Moisture Content (%)	Dry Density (lbs/cu.ft.)	Depth (ft.)	Soil Type	Description Surface Conditions: Parking Lot
				30		
				31		
				32		
				33		
				34		
				35		
				36		
				37		
				38		
				39		
				40		
				41		
				42		
				43		
				44		
				45		
				46		
				47		Total Depth: 41.5' No Groundwater Fill Suspected to 16 feet
				48		
				49		
				50		
				51		
				52		
				53		
				54		
				55		
				56		
				57		
				58		
				59		
				60		

RYBAK GEOTECHNICAL, INC.

BORING NO. 9

Client: AVERY COMPANY
Project No. 1795

Exploration Date: August 25, 2007

Sample Depth (ft.)	Blows per foot	Moisture Content (%)	Dry Density (lbs/cu.ft.)	Depth (ft.)	Soil Type	Description Surface Conditions: Parking Lot - 3 inch thick A.C.
				0		
				1		EXISTING FILL: Silty Sand, brown, moist, medium dense, some slight brick fragments
				2		
				3		
				4	SM	NATURAL SOILS: Silty Sand, brown, moist, fine-grained
5	23	3.5	115.3	5		
				6		
				7		
				8		
				9		sandier
10	40	11.7	116	10		
				11		
				12		
				13	SP	Gravelly Sand, brown, moist, medium to coarse-grained, friable
				14		
15	50 for 6"	14	109.8	15		
				16		
				17	SM	Silty Sand, brown, dense, fine-grained
				18		
				19		
20	40	16.7	112.3	20		
				21		
				22		
				23		Total Depth: 21.5' No Groundwater Fill to 3.5 feet
				24		
				25		
				26		
				27		
				28		
				29		
				30		

RYBAK GEOTECHNICAL, INC.

BORING NO. 10

Client: AVERY COMPANY

Exploration Date: August 25, 2007

Project No. 1795

Sample Depth, (ft.)	Blows per foot	Moisture Content (%)	Dry Density (lbs/cu.ft.)	Depth (ft.)	Soil Type	Description Surface Conditions: Concrete Slab
				0		Two, five-inch thick concrete slabs sandwiching a 2 inch thick sand layer
				1		SUSPECTED FILL: Silty Sand, brown, moist, medium dense, fine to medium grained, some gravel
				2		
				3	SP	NATURAL SOILS: Sand, grayish yellow brown, moist, dense, medium to coarse poorly graded
				4		
5	34	2.4	--	5		
				6		
				7		
				8		
				9		
10	50 for 6"	2.4	--	10		
				11		
				12		
				13		
				14		abundant gravel and cobbles, very friable
15	50 for 6"	2.7	--	15		
				16		
				17		
				18		
				19		
20	50 for 6"	2.1	--	20		
				21		
				22		Total Depth: 21.5' No Groundwater Fill suspected to 2.5 feet
				23		
				24		
				25		
				26		
				27		
				28		
				29		
				30		

Table 1. Type and Quantity of Laboratory Test

Laboratory Test	Quantity	ASTM Standard
Dry Density	38	-----
Moisture Content	52	D-2216
Consolidation	7	D-2435
Direct Shear	4	D-3080
Compaction	2	D-1557
Expansion Index	2	D-4829
Water-Soluble Sulfate	1	California Test Method 417
Electrical Resistivity	1	California Test Method 532
pH	1	California Test Method 643
Chlorides	1	California Test Method 422

Table 2. Results of the Dry Density-Moisture Content Tests

Location	Depth ft.	Soil Description	Dry Density, pcf	Moisture Content, %
B1	2.5	Tan and orange-brown, gravelly Sand	108.5	4.7
B1	5.0	Tan and orange-brown, gravelly Sand	103.9	2.9
B1	10.0	Brown, silty Sand	108.7	1.9
B1	15.0	Gold-brown, Sand	101.0	11.0
B1	20.0	Brown, sandy Silt	107.6	12.8
B1	25.0	Brown to orange-brown, gravelly Sand	-----	1.8
B1	30.0	Brown to orange-brown, gravelly Sand	108.6	3.0
B2	2.5	Brown, Sand	116.8	3.0
B2	5.0	Brown, Sand	-----	2.8
B2	10.0	Gold-brown, Sand with silt	107.2	3.4
B2	15.0	Light brown, Sand	-----	1.9
B3	2.5	Brown, Sand	107.5	3.3

Table 2. Results of the Dry Density-Moisture Content Tests (Continued)

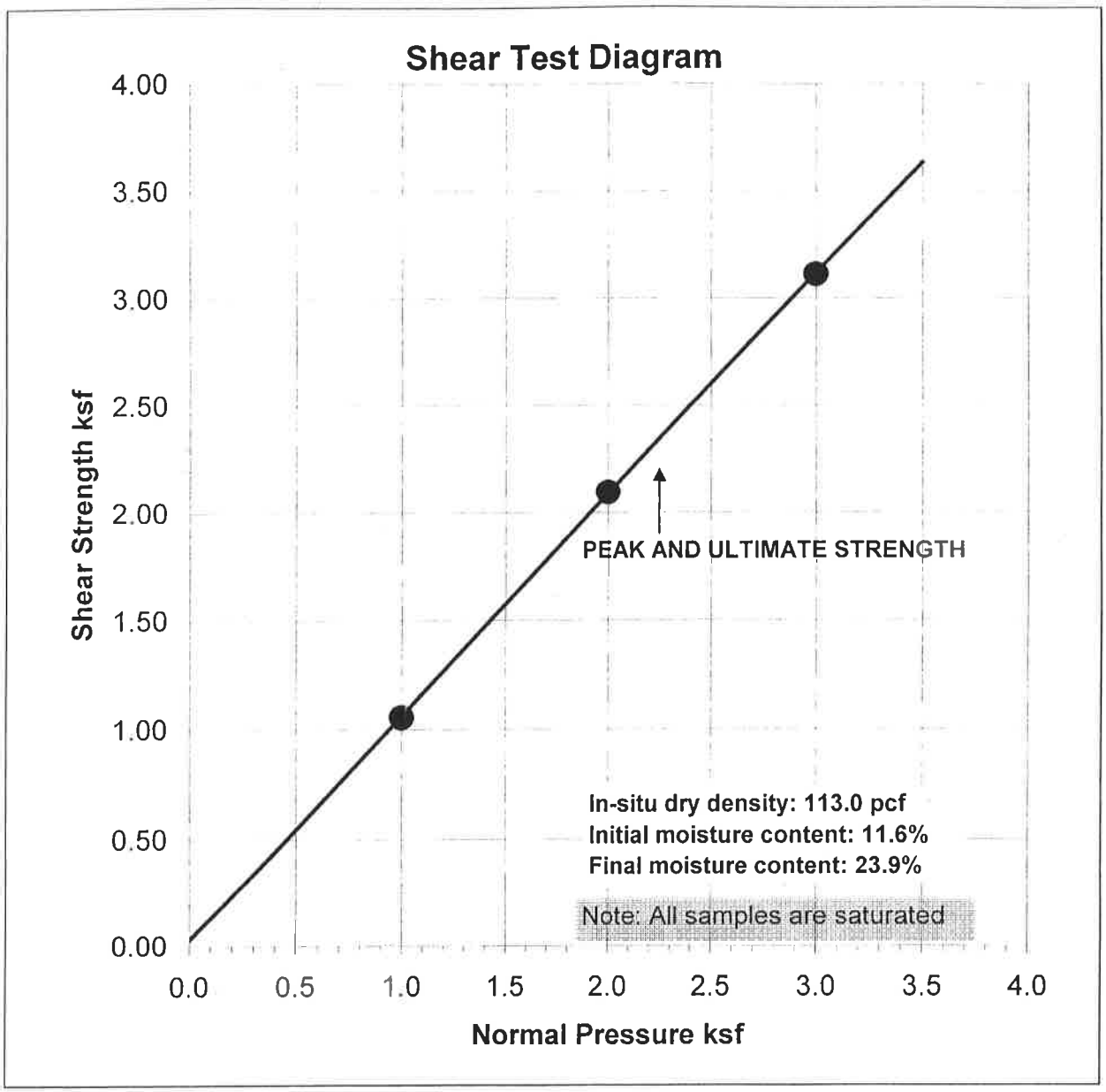
Location	Depth ft.	Soil Description	Dry Density, pcf	Moisture Content, %
B3	5.0	Gold-brown, Sand	104.0	2.7
B3	10.0	Gold-brown, Sand with gravel	103.1	5.0
B3	15.0	Gold-brown, Sand with gravel	110.7	5.5
B3	20.0	Gold-brown, Sand with gravel	105.3	3.5
B4	2.5	Brown, silty Sand	104.5	2.2
B4	5.0	Brown, silty Sand	93.9	8.4
B4	10.0	Brown, silty Sand	102.2	8.5
B4	15.0	Brown, silty Sand	112.1	8.9
B4	20.0	Brown, silty Sand	-----	1.8
B4	25.0	Brown to gold-brown, Sand	-----	1.4
B5	2.5	Gold-brown, Sand	117.2	3.7
B5	5.0	Gold-brown, Sand	107.2	4.3
B5	10.0	Brown, silty Sand	107.1	10.1
B5	15.0	Brown, silty Sand	116.7	7.2
B5	20.0	Brown, silty Sand	108.7	6.9
B5	25.0	Brown, silty Sand	-----	7.9
B6	2.5	Brown, Sand	117.6	2.5
B6	5.0	Brown, silty Sand	105.5	3.3
B6	10.0	Gold-brown, Sand	102.2	13.3
B6	15.0	Brown-white, Sand	-----	3.5
B6	20.0	Brown, silty Sand	111.8	4.6
B6	25.0	Gold-brown, Sand	105.6	2.6
B6	30.0	Gold-brown, Sand	113.4	2.0
B7	5.0	Light to dark tan, gravelly Sand	105.0	9.8
B7	10.0	Dark orange-brown, Sand with silt	-----	4.2

Table 2. Results of the Dry Density-Moisture Content Tests (Continued)

Location	Depth ft.	Soil Description	Dry Density, pcf	Moisture Content, %
B7	15.0	Dark brown/mottled tan-brown, Sand with silt	116.3	4.2
B7	20.0	Dark orange-brown, Sand with silt	-----	4.1
B8	5.0	Dark brown, silty Sand	120.7	8.3
B8	10.0	Dark yellow-brown, silty Sand	112.5	7.6
B8	15.0	Gray, silty Sand	109.9	13.4
B8	20.0	Dark golden tan, Sand with silt	110.9	7.6
B9	5.0	Light to dark tan, gravelly Sand	115.3	3.5
B9	10.0	Brown, silty Sand	116.0	11.7
B9	15.0	Gold brown mottled black, Sand	80.0	4.4
B9	20.0	Brown, clayey silty Sand	112.3	16.7
B10	5.0	Gray yellow-brown, Sand	-----	2.4
B10	10.0	Light to dark golden tan, Sand with silt	-----	2.4
B10	15.0	Light to dark golden tan, gravelly Sand with silt	-----	2.7
B10	20.0	Golden tan, Sand with silt	-----	2.1

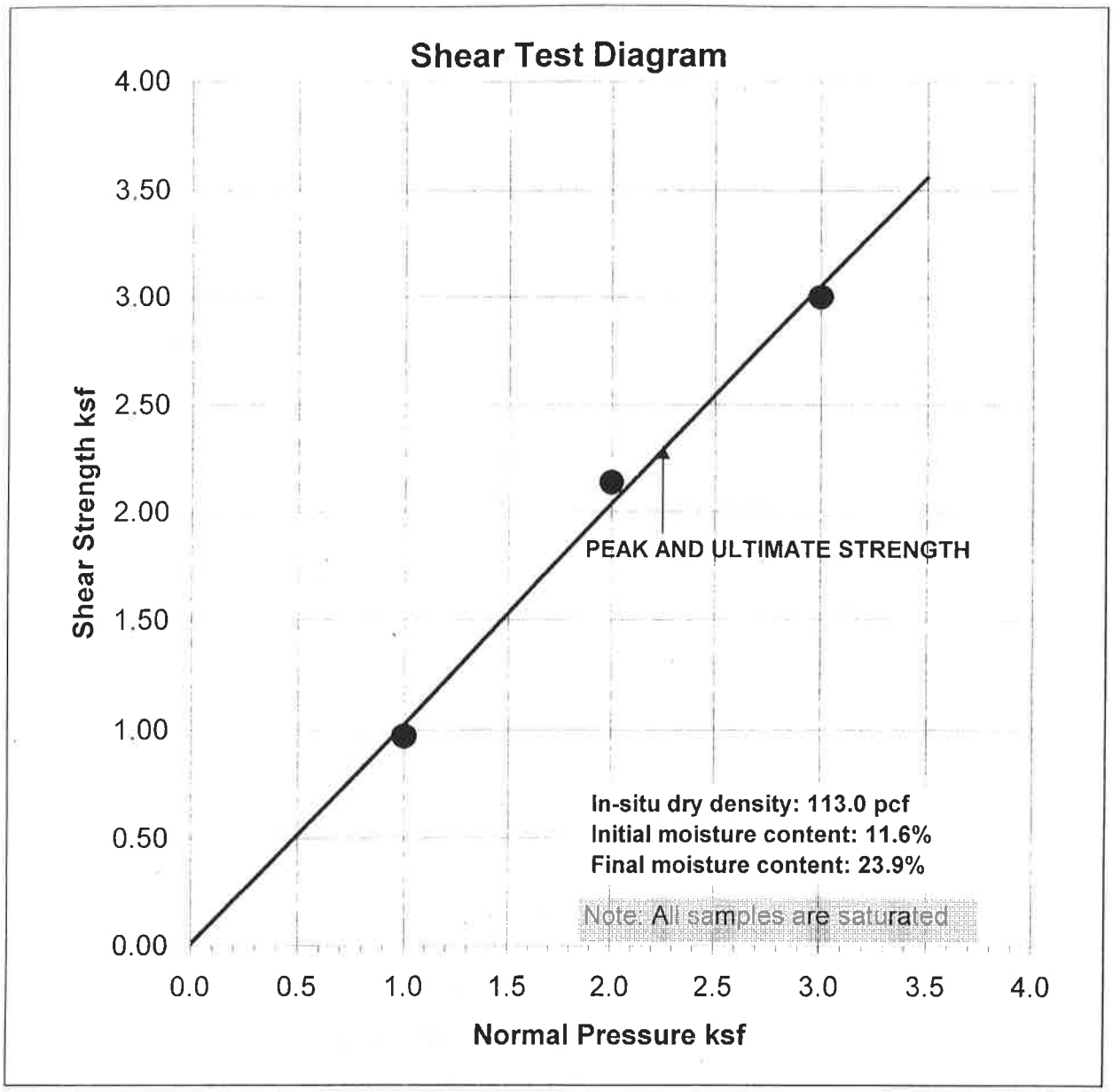
Table 3. Results of Laboratory Compaction Tests and Expansion Index Test

Location	Depth ft.	Soil Description	Maximum Dry Density, pcf	Optimum Moisture, %	Expansion Index
B5	0-5	Gold-brown, Sand	128.0	9.0	1 - Low
B6	0-5	Brown, silty Sand	124.0	9.0	3 - Low



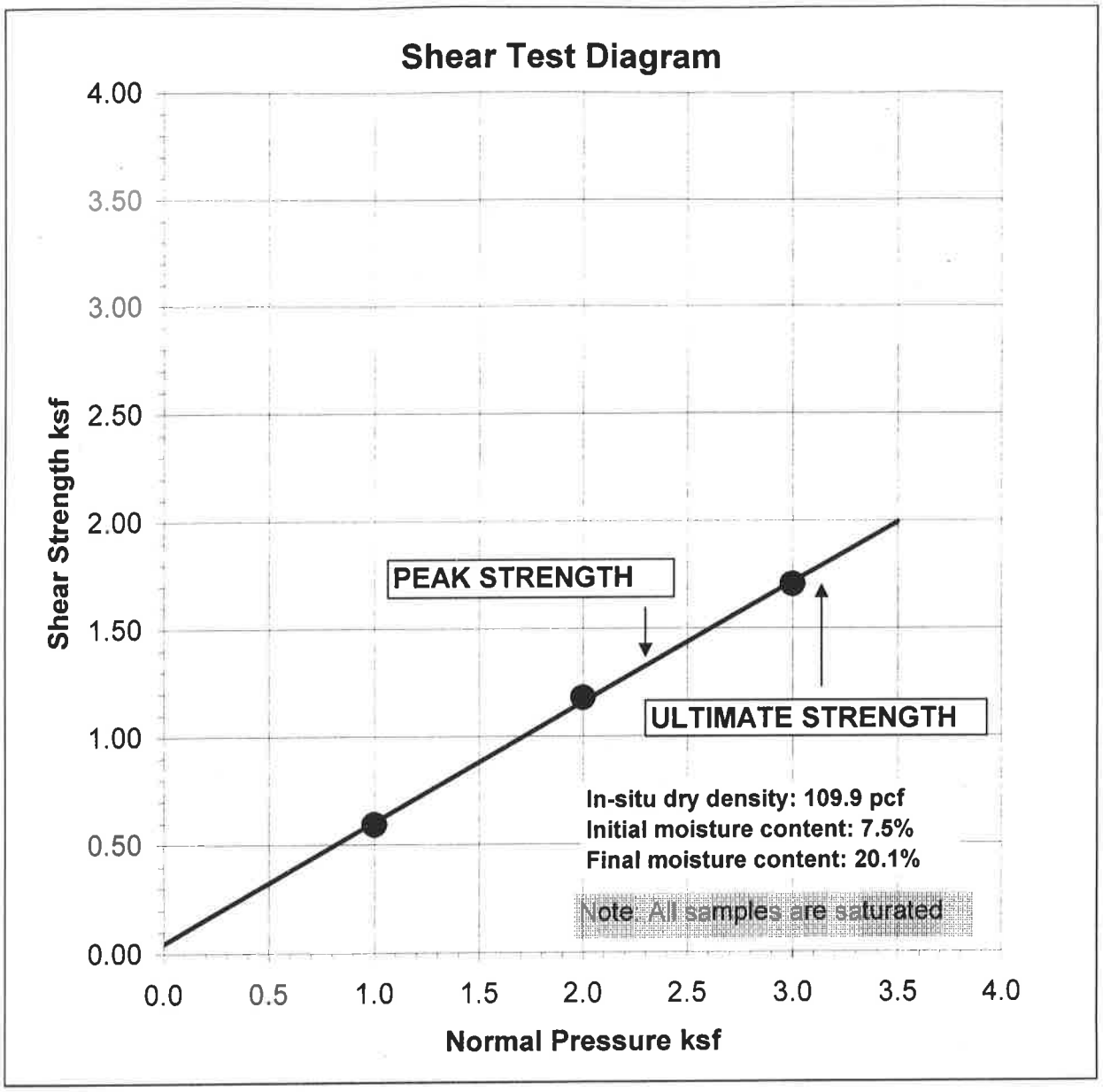
DATE: 7/17/2006	SAMPLE: B2 - 2.5'
CLIENT: AVERY	PROJECT NO. 1795

RYBAK GEOTECHNICAL, INC.



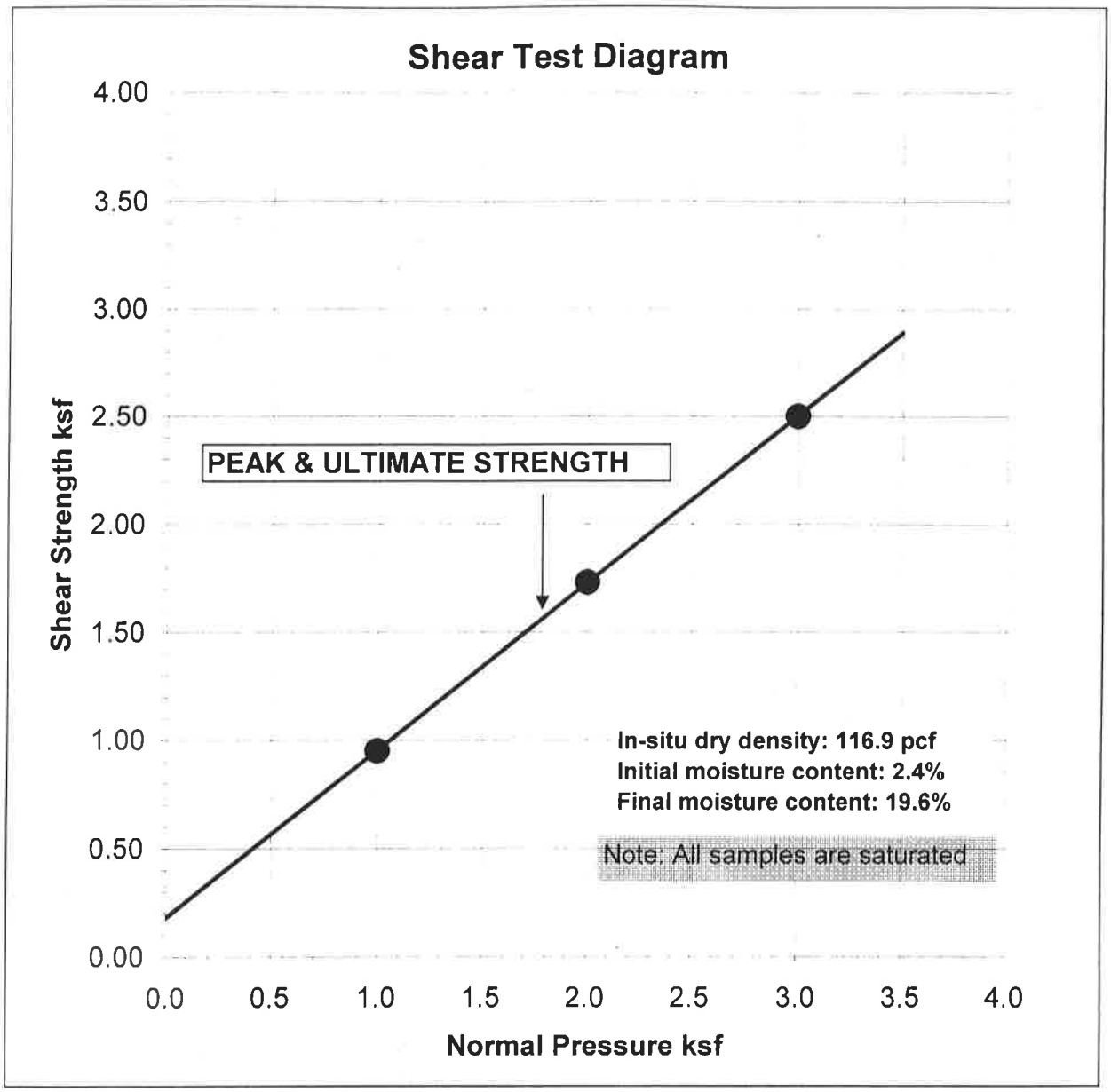
DATE: 7/17/2006	SAMPLE: B3 - 5'
CLIENT: AVERY	PROJECT NO. 1795

RYBAK GEOTECHNICAL, INC.



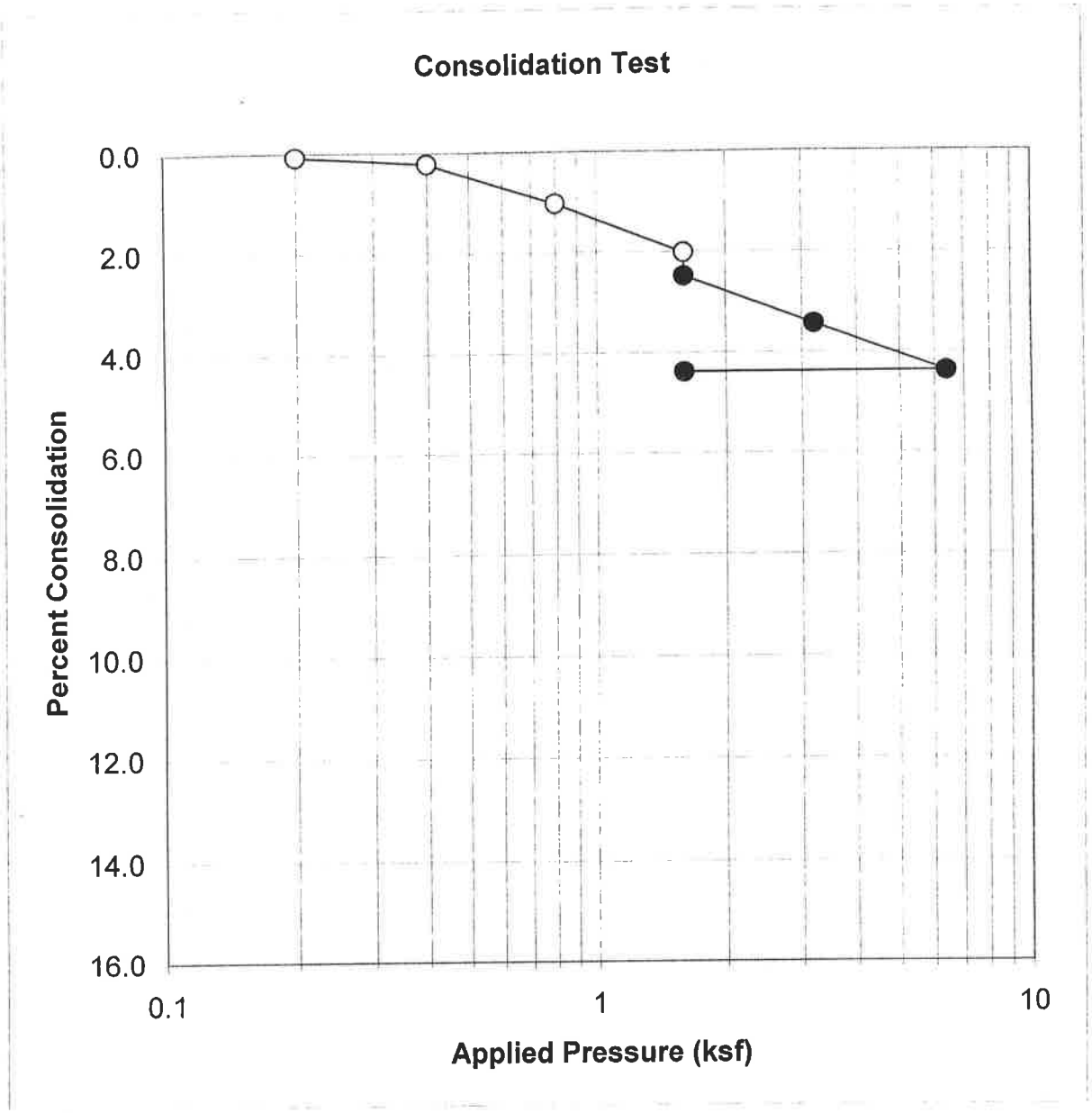
DATE: 9/6/2007	SAMPLE: B8 - 10'
CLIENT: AVERY	PROJECT NO. 1795

RYBAK GEOTECHNICAL, INC.



DATE: 9/6/2007	SAMPLE: B10 - 15'
CLIENT: AVERY	PROJECT NO. 1795

RYBAK GEOTECHNICAL, INC.

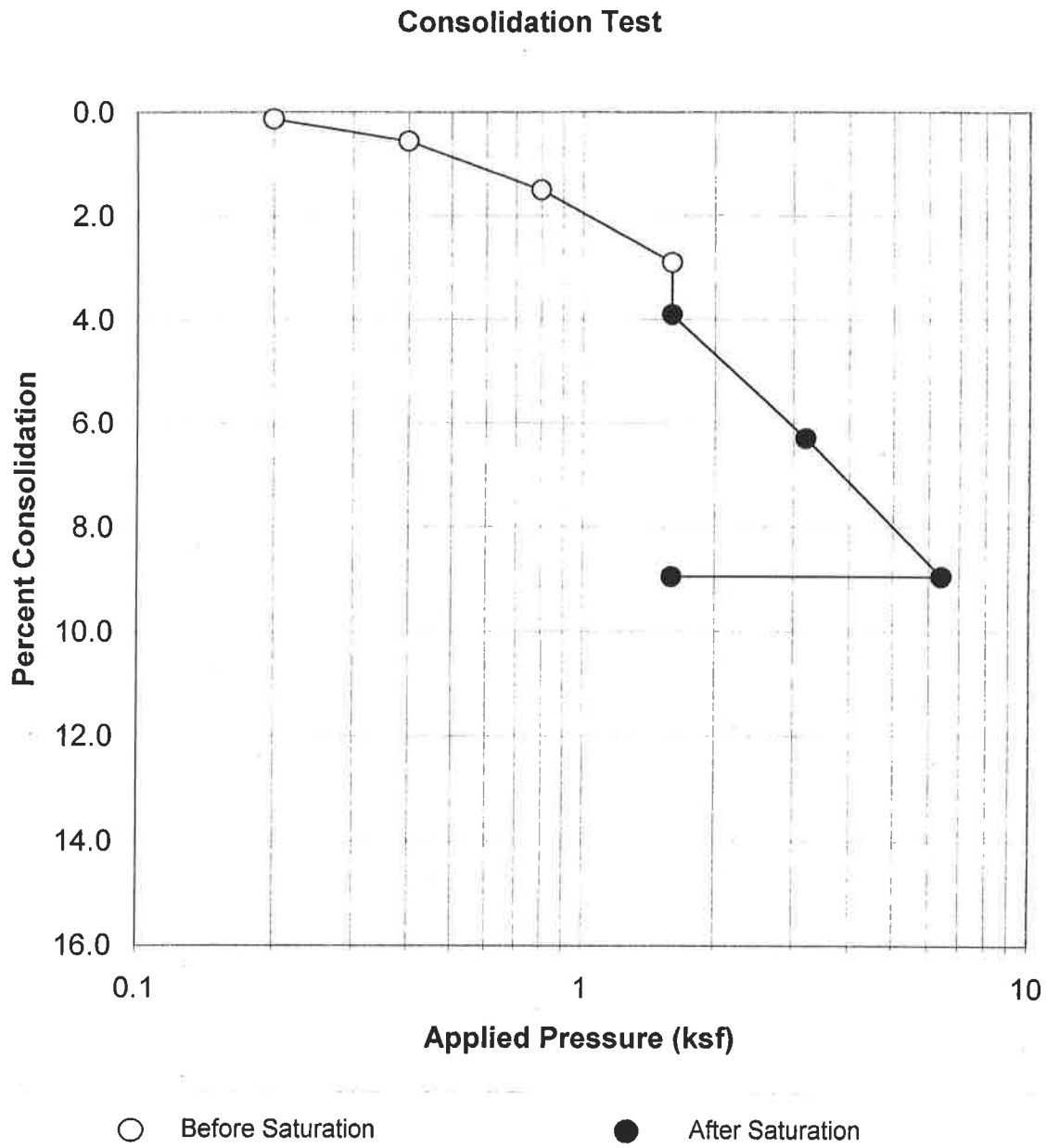


○ Before Saturation ● After Saturation

DATE: 7/17/2006	SAMPLE: B3 - 2.5'
CLIENT: AVERY	PROJECT NO. 1795

RYBAK GEOTECHNICAL, INC.

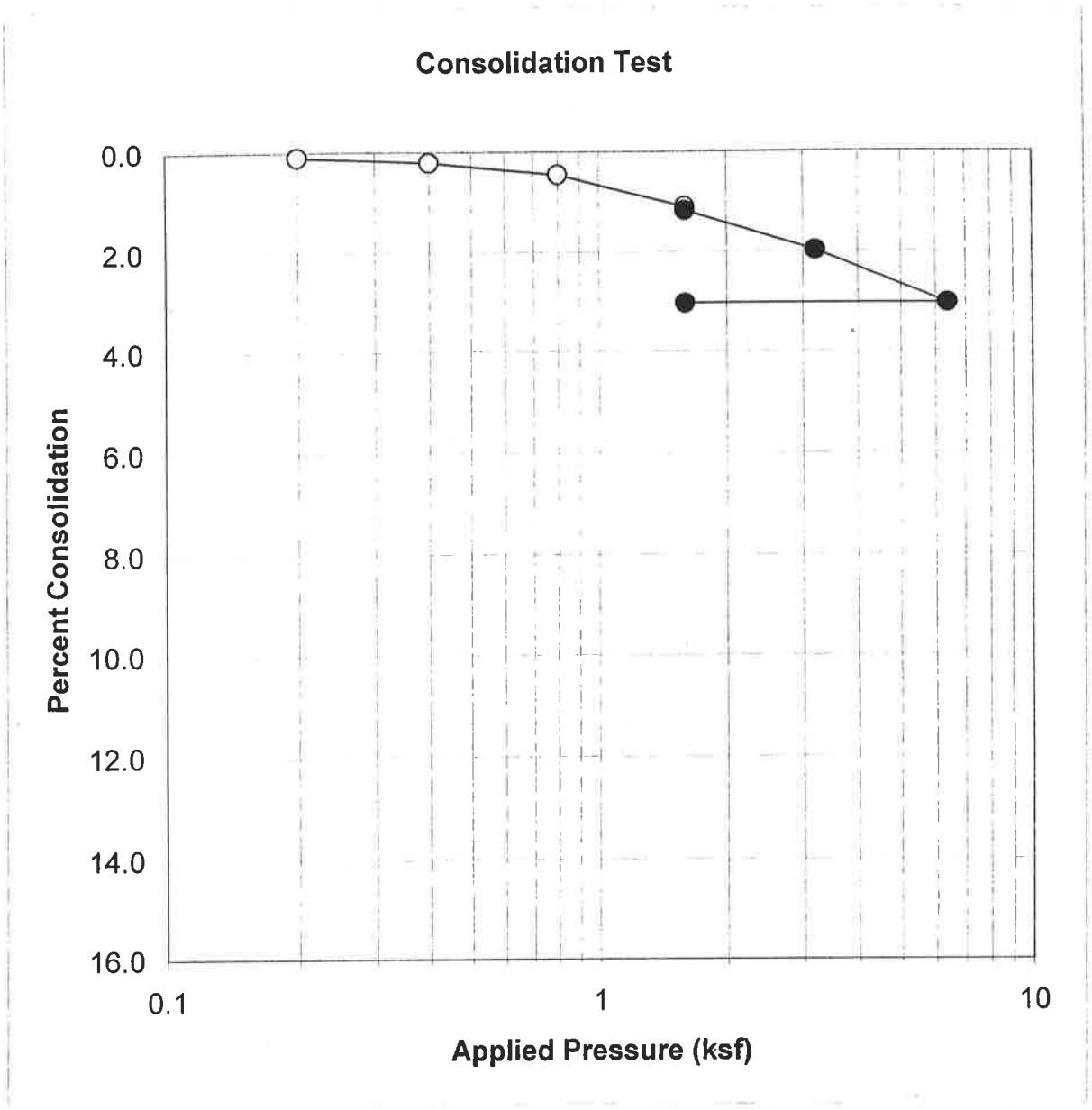
Plate C-1



DATE: 7/17/2006	SAMPLE: B4 - 5'
CLIENT: AVERY	PROJECT NO. 1795

RYBAK GEOTECHNICAL, INC.

Plate C-2

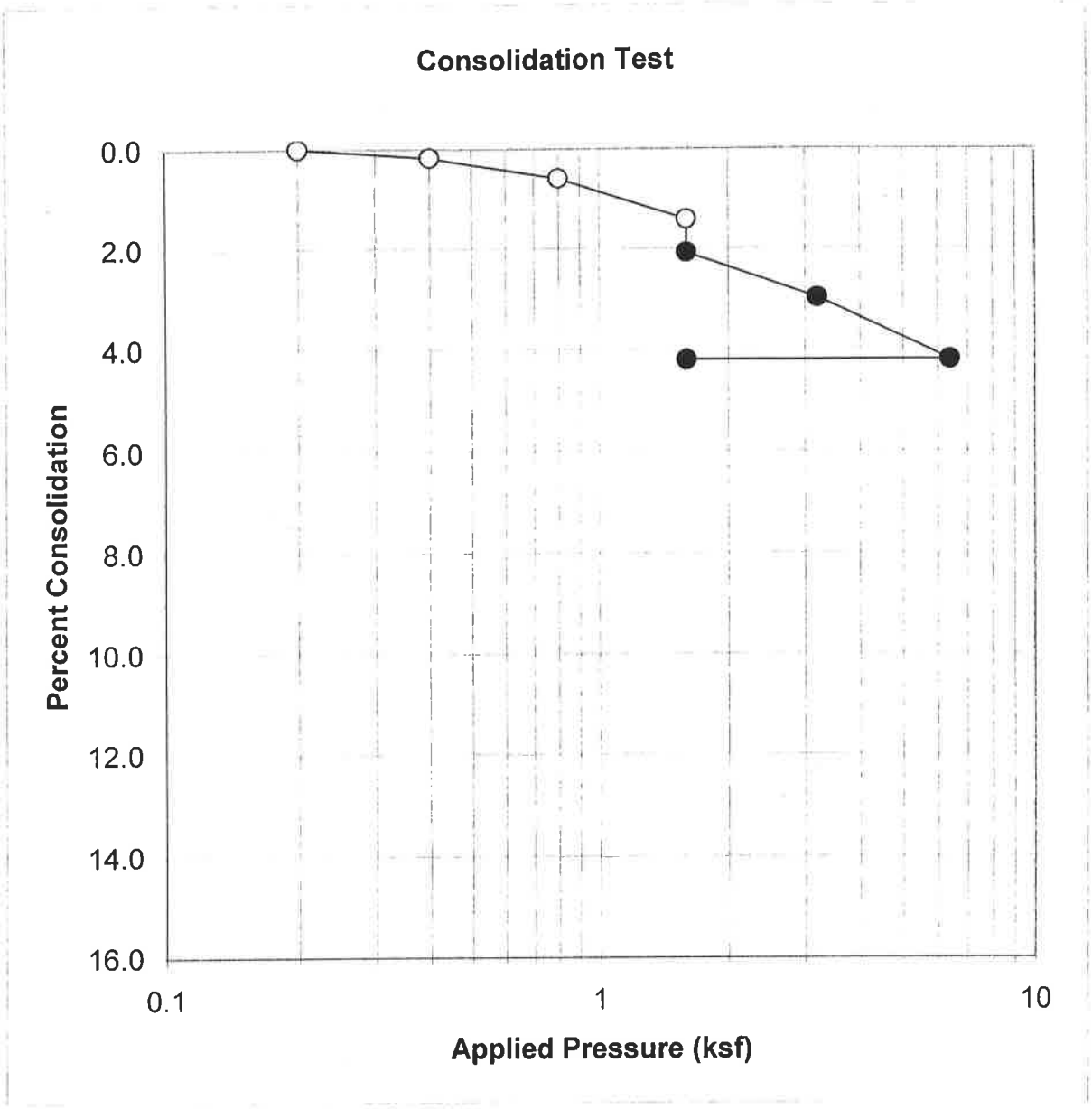


○ Before Saturation ● After Saturation

DATE: 7/17/2006	SAMPLE: B1 - 10'
CLIENT: AVERY	PROJECT NO. 1795

RYBAK GEOTECHNICAL, INC.

Plate C-3

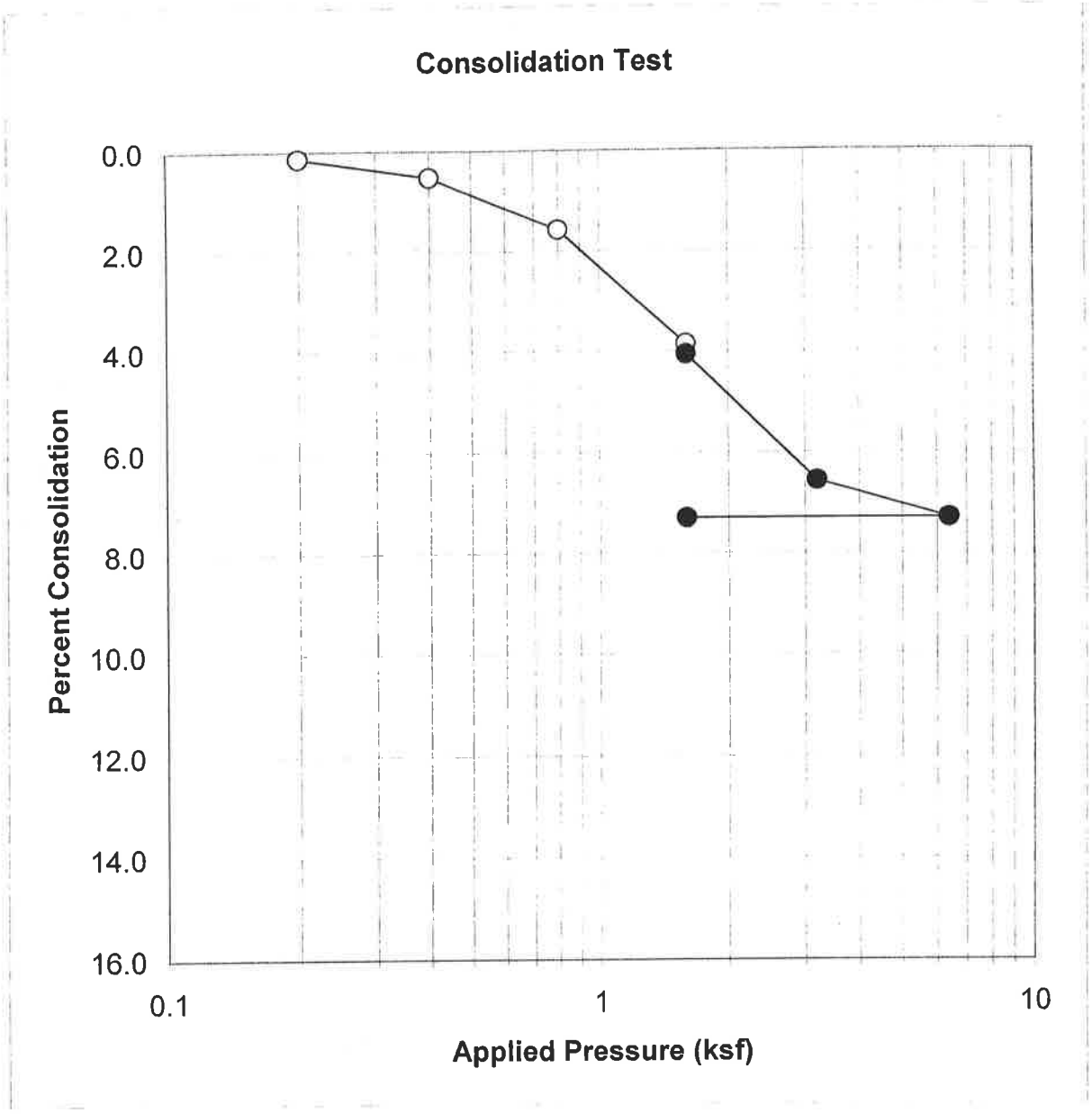


○ Before Saturation ● After Saturation

DATE: 7/17/2006	SAMPLE: B4 - 15'
CLIENT: AVERY	PROJECT NO. 1795

RYBAK GEOTECHNICAL, INC.

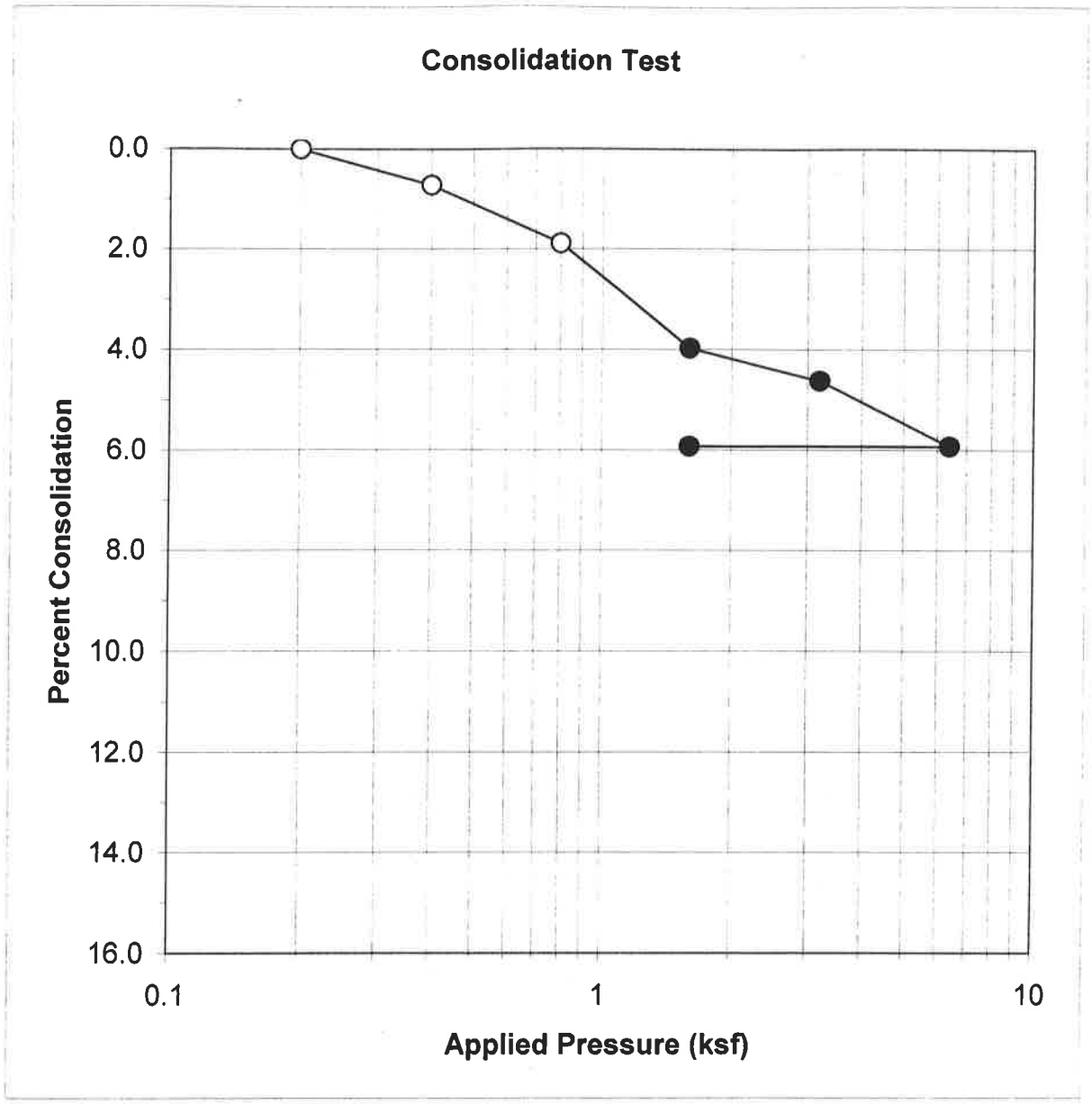
Plate C-4



○ Before Saturation ● After Saturation

DATE: 7/17/2006	SAMPLE: B1 - 20'
CLIENT: AVERY	PROJECT NO. 1795

RYBAK GEOTECHNICAL, INC.

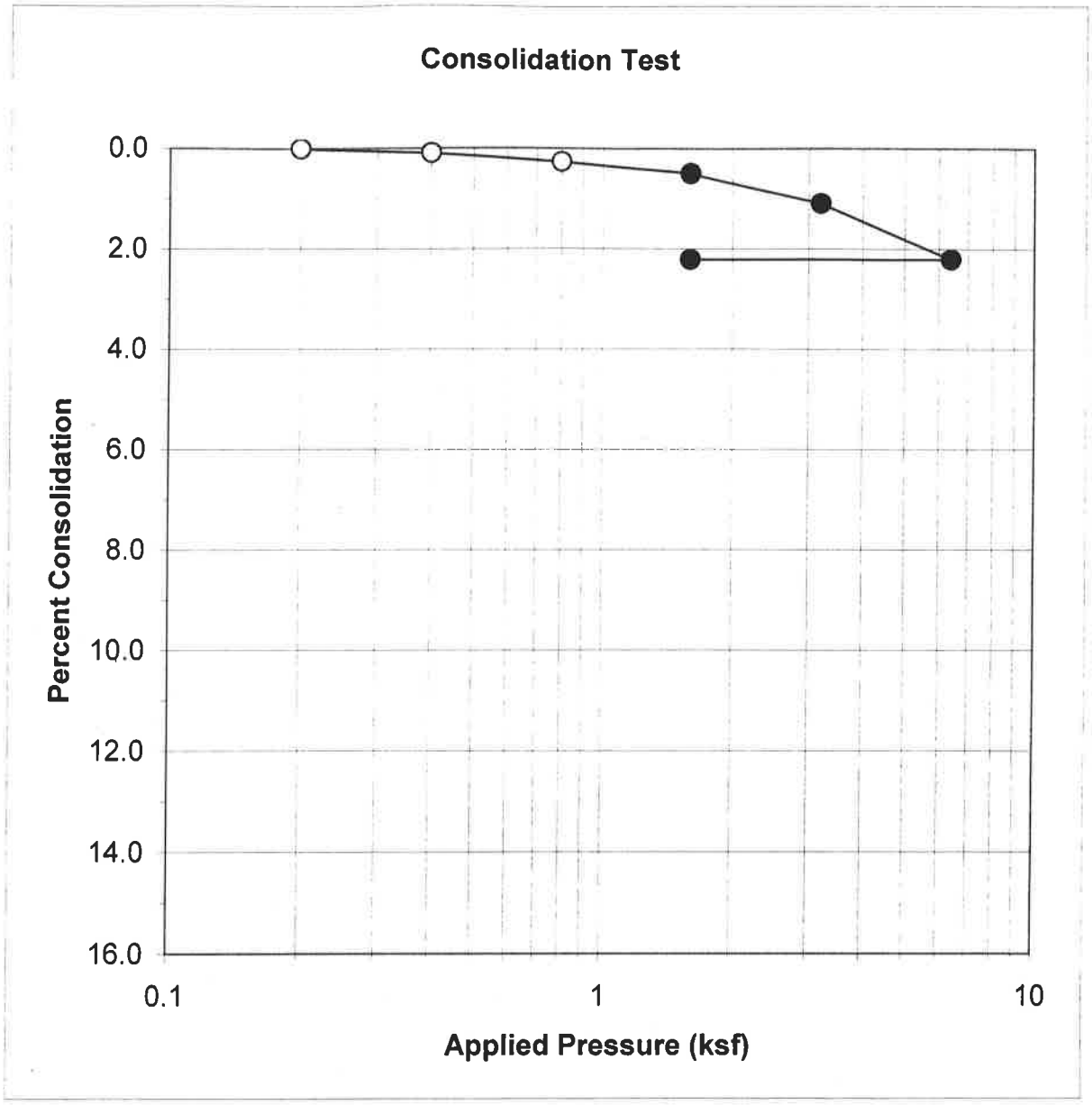


○ Before Saturation ● After Saturation

DATE: 9/10/2007	SAMPLE: B9 - 10'
CLIENT: AVERY	PROJECT NO. 1795

RYBAK GEOTECHNICAL, INC.

Plate C-6



○ Before Saturation ● After Saturation

DATE: 9/10/2007	SAMPLE: B7 - 15'
CLIENT: AVERY	PROJECT NO. 1795

RYBAK GEOTECHNICAL, INC.

Plate C-7

AP Engineering and Testing, Inc.

Geotechnical Testing Laboratory

CORROSION TEST RESULTS

Client Name: Rybak Geotechnical
Project Name: Avery
Project No.: 1795

AP Job No.: 26-0751
Date 07/18/06

Boring No.	Sample No.	Depth (ft)	Visual Soil Type	Minimum Resistivity (ohm-cm)	pH	Sulfate Content (ppm)	Chloride Content (ppm)
				SATURATED			
B5	-	0-5	SP-SM	4700	7.6	24	61
B6	-	0-5	SP-SM	5200	7.9	17	61

NOTES:
Resistivity Test and pH: California Test Methods 532 and 643
Sulfate Content : California Test Method 417
Chloride Content : California Test Method 422
ND = Not Detectable
NA = Not Sufficient Sample
NR = Not Requested

2307 Pomona Boulevard, Pomona, CA 91768
Tel. (909) 869-6316 Fax. (909)869-6318

PLATE D

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*    Version 3.00    *
*                    *
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DETERMINISTIC ESTIMATION OF
PEAK ACCELERATION FROM DIGITIZED FAULTS

JOB NUMBER: 1795

DATE: 07-15-2006

JOB NAME: Avery Company

CALCULATION NAME: Avery Company

FAULT-DATA-FILE NAME: CDMGFLTE.DAT

SITE COORDINATES:
SITE LATITUDE: 34.3806
SITE LONGITUDE: 118.5292

SEARCH RADIUS: 62 mi

ATTENUATION RELATION: 14) Campbell & Bozorgnia (1997 Rev.) - Alluvium
UNCERTAINTY (M=Median, S=Sigma): S Number of Sigmas: 1.0
DISTANCE MEASURE: cdist
SCOND: 0
Basement Depth: 5.00 km Campbell SSR: 0 Campbell SHR: 0
COMPUTE PEAK HORIZONTAL ACCELERATION

FAULT-DATA FILE USED: CDMGFLTE.DAT

MINIMUM DEPTH VALUE (km): 3.0

avry.OUT

EQFAULT SUMMARY

DETERMINISTIC SITE PARAMETERS

Page 1

ABBREVIATED FAULT NAME	APPROXIMATE DISTANCE mi (km)	ESTIMATED MAX. EARTHQUAKE EVENT MAXIMUM EARTHQUAKE MAG.(Mw)	PEAK SITE ACCEL. g	EST. SITE INTENSITY MOD.MERC.
HOLSER	2.9(4.7)	6.5	0.900	XI
SAN GABRIEL	3.0(4.9)	7.2	0.717	XI
SANTA SUSANA	3.5(5.7)	6.7	0.858	XI
NORTHRIDGE (E. Oak Ridge)	4.3(7.0)	7.0	0.823	XI
SIERRA MADRE (San Fernando)	4.8(7.7)	6.7	0.749	XI
VERDUGO	10.1(16.2)	6.9	0.469	X
OAK RIDGE (Onshore)	11.2(18.1)	7.0	0.443	X
SIERRA MADRE	14.3(23.0)	7.2	0.388	X
SAN CAYETANO	14.7(23.7)	7.0	0.336	IX
SIMI-SANTA ROSA	16.7(26.8)	7.0	0.295	IX
HOLLYWOOD	20.1(32.3)	6.4	0.169	VIII
SAN ANDREAS - 1857 Rupture	20.7(33.3)	7.8	0.372	IX
SAN ANDREAS - Mojave	20.7(33.3)	7.4	0.289	IX
SANTA MONICA	21.1(33.9)	6.6	0.181	VIII
UPPER ELYSIAN PARK	21.3(34.3)	6.4	0.157	VIII
PUENTE HILLS BLIND THRUST	22.2(35.7)	7.1	0.232	IX
SAN ANDREAS - Carrizo	22.2(35.8)	7.4	0.270	IX
MALIBU COAST	23.2(37.4)	6.7	0.172	VIII
ANACAPA-DUME	24.4(39.3)	7.5	0.264	IX
RAYMOND	24.5(39.4)	6.5	0.143	VIII
SANTA YNEZ (East)	25.5(41.1)	7.1	0.194	VIII
NEWPORT-INGLEWOOD (L.A.Basin)	25.7(41.3)	7.1	0.193	VIII
CLAMSHELL-SAWPIT	28.0(45.0)	6.5	0.122	VII
PALOS VERDES	30.0(48.2)	7.3	0.190	VIII
VENTURA - PITAS POINT	35.2(56.7)	6.9	0.121	VII
GARLOCK (West)	37.0(59.5)	7.1	0.132	VIII
M.RIDGE-ARROYO PARIDA-SANTA ANA	37.5(60.3)	7.2	0.137	VIII
PLEITO THRUST	37.9(61.0)	7.0	0.118	VII
WHITTIER	40.0(64.3)	6.8	0.094	VII
BIG PINE	40.9(65.8)	6.9	0.099	VII
SAN JOSE	42.8(68.8)	6.5	0.068	VI
CHANNEL IS. THRUST (Eastern)	43.6(70.1)	7.5	0.139	VIII
OAK RIDGE MID CHANNEL STRUCTURE	43.7(70.3)	6.6	0.071	VI
RED MOUNTAIN	43.7(70.4)	7.0	0.097	VII
OAK RIDGE(Blind Thrust Offshore)	44.1(70.9)	7.1	0.104	VII
CUCAMONGA	44.3(71.3)	6.9	0.088	VII
CHINO-CENTRAL AVE. (Elsinore)	49.8(80.2)	6.7	0.064	VI
WHITE WOLF	52.0(83.7)	7.3	0.097	VII
SAN ANDREAS - Southern	57.5(92.5)	7.4	0.099	VII
SAN ANDREAS - San Bernardino	57.5(92.5)	7.5	0.108	VII

DETERMINISTIC SITE PARAMETERS

Page 2

ABBREVIATED FAULT NAME	APPROXIMATE DISTANCE mi (km)	ESTIMATED MAX. EARTHQUAKE EVENT MAXIMUM EARTHQUAKE MAG.(Mw)	PEAK SITE ACCEL. g	EST. SITE INTENSITY MOD.MERC.
SAN JACINTO-SAN BERNARDINO	59.0(95.0)	6.7	0.052	VI
CLEGHORN	61.0(98.1)	6.5	0.041	V

-END OF SEARCH- 42 FAULTS FOUND WITHIN THE SPECIFIED SEARCH RADIUS.

THE HOLSER FAULT IS CLOSEST TO THE SITE.
IT IS ABOUT 2.9 MILES (4.7 km) AWAY.

LARGEST MAXIMUM-EARTHQUAKE SITE ACCELERATION: 0.8996 g

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ESTIMATION OF
PEAK ACCELERATION FROM
CALIFORNIA EARTHQUAKE CATALOGS

JOB NUMBER: 1795

DATE: 07-15-2006

JOB NAME: Avery Company

EARTHQUAKE-CATALOG-FILE NAME: ALLQUAKE.DAT

MAGNITUDE RANGE:
MINIMUM MAGNITUDE: 4.00
MAXIMUM MAGNITUDE: 9.00

SITE COORDINATES:
SITE LATITUDE: 34.3806
SITE LONGITUDE: 118.5292

SEARCH DATES:
START DATE: 1800
END DATE: 2006

SEARCH RADIUS:
62.0 mi
99.8 km

ATTENUATION RELATION: 14) Campbell & Bozorgnia (1997 Rev.) - Alluvium
UNCERTAINTY (M=Median, S=Sigma): S Number of Sigmas: 1.0
ASSUMED SOURCE TYPE: DS [SS=Strike-slip, DS=Reverse-slip, BT=Blind-thrust]
SCOND: 0 Depth Source: A
Basement Depth: 5.00 km Campbell SSR: 0 Campbell SHR: 0
COMPUTE PEAK HORIZONTAL ACCELERATION

MINIMUM DEPTH VALUE (km): 3.0

EARTHQUAKE SEARCH RESULTS

FILE CODE	LAT. NORTH	LONG. WEST	DATE	TIME (UTC) H M Sec	DEPTH (km)	QUAKE MAG.	SITE ACC. g	SITE MM INT.	APPROX. DISTANCE mi [km]
GSP	34.3790	118.5610	01/18/1994	152346.9	7.0	4.80	0.304	IX	1.8(2.9)
GSP	34.3790	118.5630	01/18/1994	003935.0	7.0	4.40	0.236	IX	1.9(3.1)
GSP	34.3740	118.4950	01/28/1994	200953.4	0.0	4.20	0.207	VIII	2.0(3.2)
GSG	34.4080	118.5590	01/17/1994	200205.4	0.0	4.00	0.172	VIII	2.5(4.1)
DMG	34.3610	118.4870	02/10/1971	143526.7	4.4	4.20	0.191	VIII	2.8(4.4)
GSB	34.3600	118.5710	01/19/1994	044048.0	2.0	4.50	0.231	IX	2.8(4.5)
GSB	34.3450	118.5520	01/24/1994	041518.8	6.0	4.80	0.277	IX	2.8(4.5)
GSP	34.3570	118.4800	02/25/1994	125912.6	1.0	4.10	0.169	VIII	3.2(5.2)
DMG	34.3990	118.4730	03/09/1974	05431.9	24.4	4.70	0.242	IX	3.4(5.5)
DMG	34.3560	118.4740	03/25/1971	2254 9.9	4.6	4.20	0.173	VIII	3.6(5.7)
PAS	34.3800	118.4590	08/12/1977	21926.1	9.5	4.50	0.199	VIII	4.0(6.4)
GSG	34.3340	118.4840	01/17/1994	223152.1	10.0	4.20	0.162	VIII	4.1(6.6)
DMG	34.3840	118.4550	02/10/1971	113134.6	6.0	4.20	0.159	VIII	4.2(6.8)
GSB	34.3190	118.5580	01/18/1994	132444.1	1.0	4.50	0.186	VIII	4.6(7.3)
DMG	34.3530	118.4560	03/07/1971	13340.5	3.3	4.50	0.185	VIII	4.6(7.4)
GSP	34.3970	118.6090	07/22/1999	095724.0	11.0	4.00	0.131	VIII	4.7(7.5)
GSP	34.3620	118.6150	03/20/1996	073759.8	13.0	4.10	0.134	VIII	5.1(8.1)
GSP	34.3780	118.6180	01/19/1994	211144.9	11.0	5.10	0.258	IX	5.1(8.1)
DMG	34.3970	118.4390	02/21/1971	55052.6	6.9	4.70	0.193	VIII	5.3(8.5)
GSP	34.3740	118.6220	01/17/1994	155410.8	12.0	4.80	0.205	VIII	5.3(8.5)
DMG	34.3800	118.6230	10/29/1936	223536.1	10.0	4.00	0.120	VII	5.3(8.6)
GSB	34.3580	118.6220	01/18/1994	040126.8	1.0	4.50	0.164	VIII	5.5(8.9)
GSP	34.3630	118.6270	01/24/1994	055421.1	10.0	4.20	0.131	VIII	5.7(9.2)
GSB	34.3100	118.4740	01/21/1994	184228.8	7.0	4.20	0.130	VIII	5.8(9.3)
GSB	34.3010	118.5650	01/17/1994	204602.4	9.0	5.20	0.253	IX	5.9(9.4)
DMG	34.3920	118.4270	02/21/1971	71511.7	7.2	4.50	0.157	VIII	5.9(9.4)
GSP	34.3590	118.6290	01/24/1994	055024.3	12.0	4.30	0.137	VIII	5.9(9.5)
GSP	34.3050	118.5790	01/29/1994	112036.0	1.0	5.10	0.232	IX	5.9(9.6)
GSP	34.3310	118.4420	01/17/1994	141430.3	1.0	4.50	0.154	VIII	6.0(9.7)
GSP	34.3170	118.4550	01/17/1994	132644.7	2.0	4.70	0.174	VIII	6.1(9.8)
GSP	34.3040	118.4730	01/17/1994	150703.2	2.0	4.20	0.124	VII	6.2(9.9)
GSP	34.3680	118.6370	01/17/1994	194353.4	13.0	4.10	0.115	VII	6.2(10.0)
GSB	34.3330	118.6230	01/18/1994	072356.0	14.0	4.30	0.131	VIII	6.3(10.1)
GSP	34.3110	118.4560	01/17/1994	193534.3	2.0	4.00	0.104	VII	6.4(10.2)
DMG	34.3990	118.4190	02/10/1971	134953.7	9.7	4.30	0.129	VIII	6.4(10.3)
DMG	34.2860	118.5150	03/31/1971	145222.5	2.1	4.60	0.154	VIII	6.6(10.6)
DMG	34.3440	118.6360	02/09/1971	143436.1	-2.0	4.90	0.186	VIII	6.6(10.6)
DMG	34.3080	118.4540	02/09/1971	144346.7	6.2	5.20	0.231	IX	6.6(10.6)
GSB	34.3000	118.4660	01/21/1994	183915.3	10.0	4.70	0.163	VIII	6.6(10.7)
DMG	34.2840	118.5280	04/02/1971	54025.0	3.0	4.00	0.099	VII	6.7(10.7)
GSP	34.3770	118.6490	04/27/1997	110928.4	15.0	4.80	0.170	VIII	6.8(11.0)
DMG	34.3000	118.6000	04/04/1893	1940 0.0	0.0	6.00	0.409	X	6.9(11.1)

GSP	34.2910	118.4760	02/06/1994	131926.9	11.0	4.10	0.104	VII	6.9(11.1)
DMG	34.2960	118.4640	03/30/1971	85443.3	2.6	4.10	0.104	VII	6.9(11.1)
DMG	34.4460	118.4360	02/10/1971	185441.7	8.1	4.20	0.111	VII	7.0(11.2)
GSP	34.3010	118.4520	01/21/1994	185244.2	7.0	4.30	0.119	VII	7.0(11.3)
GSP	34.2970	118.4580	01/21/1994	185344.6	7.0	4.30	0.119	VII	7.1(11.3)
GSP	34.2920	118.4660	01/19/1994	144635.2	6.0	4.00	0.093	VII	7.1(11.4)
DMG	34.3570	118.4060	02/09/1971	141950.2	11.8	4.00	0.092	VII	7.2(11.6)
DMG	34.4850	118.5210	07/16/1965	74622.4	15.1	4.00	0.092	VII	7.2(11.6)
DMG	34.4260	118.4140	02/10/1971	518 7.2	5.8	4.50	0.132	VIII	7.3(11.7)
DMG	34.4280	118.4130	04/01/1971	15 3 3.6	8.0	4.10	0.097	VII	7.4(11.9)
GSP	34.2870	118.4660	01/19/1994	071406.2	11.0	4.00	0.090	VII	7.4(11.9)

EARTHQUAKE SEARCH RESULTS

FILE CODE	LAT. NORTH	LONG. WEST	DATE	TIME (UTC) H M Sec	DEPTH (km)	QUAKE MAG.	SITE ACC. g	SITE MM INT.	APPROX. DISTANCE mi [km]
GSP	34.3610	118.6570	01/29/2002	055328.9	14.0	4.20	0.105	VII	7.4(11.9)
DMG	34.2730	118.5320	06/21/1971	16 1 8.5	4.1	4.00	0.089	VII	7.4(12.0)
PAS	34.3470	118.6560	04/08/1976	152138.1	14.5	4.60	0.136	VIII	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 133.0	8.0	4.20	0.102	VII	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 446.0	8.0	4.20	0.102	VII	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 8 7.0	8.0	4.20	0.102	VII	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 325.0	8.0	4.40	0.119	VII	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	141028.0	8.0	5.30	0.221	IX	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 140.0	8.0	4.10	0.094	VII	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 4 7.0	8.0	4.10	0.094	VII	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 159.0	8.0	4.10	0.094	VII	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 541.0	8.0	4.10	0.094	VII	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 745.0	8.0	4.50	0.127	VIII	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 550.0	8.0	4.10	0.094	VII	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 1 8.0	8.0	5.80	0.318	IX	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 8 4.0	8.0	4.00	0.087	VII	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 434.0	8.0	4.20	0.102	VII	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 853.0	8.0	4.60	0.136	VIII	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 346.0	8.0	4.10	0.094	VII	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 2 3.0	8.0	4.10	0.094	VII	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 439.0	8.0	4.10	0.094	VII	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 730.0	8.0	4.00	0.087	VII	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 244.0	8.0	5.80	0.318	IX	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 230.0	8.0	4.30	0.110	VII	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 838.0	8.0	4.50	0.127	VIII	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 444.0	8.0	4.10	0.094	VII	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 154.0	8.0	4.20	0.102	VII	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 041.8	8.4	6.40	0.466	X	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 150.0	8.0	4.50	0.127	VIII	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 231.0	8.0	4.70	0.145	VIII	7.6(12.2)
DMG	34.4110	118.4010	02/09/1971	14 710.0	8.0	4.00	0.087	VII	7.6(12.2)
GSP	34.2740	118.5630	01/27/1994	171958.8	14.0	4.60	0.136	VIII	7.6(12.2)
GSP	34.2990	118.4390	02/03/1994	162335.4	8.0	4.20	0.102	VII	7.6(12.3)
DMG	34.4570	118.4270	02/09/1971	161926.5	-1.0	4.20	0.099	VII	7.9(12.6)
GSP	34.3940	118.6690	06/26/1995	084028.9	13.0	5.00	0.169	VIII	8.0(12.9)
GSB	34.2990	118.4280	01/23/1994	085508.7	6.0	4.20	0.096	VII	8.1(13.0)
GSP	34.2690	118.5760	01/17/1994	125546.8	16.0	4.10	0.087	VII	8.1(13.1)
GSP	34.3690	118.6720	04/26/1997	103730.7	16.0	5.10	0.179	VIII	8.2(13.2)

GSB	34.3430	118.6660	01/17/1994	234925.4	8.0	4.30	0.102	VII	8.2(13.2)
GSP	34.2610	118.5340	01/17/1994	123939.8	14.0	4.50	0.118	VII	8.3(13.3)
DMG	34.4330	118.3980	02/09/1971	144017.4	-2.0	4.10	0.086	VII	8.3(13.4)
GSP	34.5000	118.5600	07/05/1991	174157.1	11.0	4.10	0.084	VII	8.4(13.6)
DMG	34.2650	118.5770	04/15/1971	111432.0	4.2	4.20	0.091	VII	8.4(13.6)
GSP	34.2780	118.6110	01/29/1994	121656.4	2.0	4.30	0.098	VII	8.5(13.6)
GSB	34.2850	118.6240	01/17/1994	135602.4	19.0	4.70	0.131	VIII	8.5(13.7)
GSP	34.2540	118.5450	01/17/1994	130627.9	0.0	4.60	0.119	VII	8.8(14.1)
GSP	34.3110	118.3980	06/15/1994	055948.6	7.0	4.20	0.086	VII	8.9(14.3)
PAS	34.4630	118.4090	09/24/1977	212824.3	5.0	4.20	0.086	VII	8.9(14.3)
GSP	34.3120	118.3930	05/25/1994	125657.1	7.0	4.40	0.098	VII	9.1(14.6)
DMG	34.2680	118.4450	08/30/1964	225737.1	15.4	4.00	0.071	VI	9.1(14.7)
DMG	34.3960	118.3660	02/10/1971	173855.1	6.2	4.20	0.081	VII	9.4(15.1)
DMG	34.3870	118.3640	02/09/1971	143917.8	-1.6	4.00	0.069	VI	9.4(15.2)
GSP	34.2890	118.4030	01/14/2001	025053.7	8.0	4.00	0.067	VI	9.6(15.4)

EARTHQUAKE SEARCH RESULTS

FILE CODE	LAT. NORTH	LONG. WEST	DATE	TIME (UTC) H M Sec	DEPTH (km)	QUAKE MAG.	SITE ACC. g	SITE MM INT.	APPROX. DISTANCE mi [km]
GSP	34.3770	118.6980	01/18/1994	004308.9	11.0	5.20	0.165	VIII	9.6(15.5)
DMG	34.4310	118.3690	08/14/1974	144555.2	8.2	4.20	0.077	VII	9.8(15.7)
GSP	34.2840	118.4040	01/14/2001	022614.1	8.0	4.30	0.084	VII	9.8(15.7)
GSP	34.2450	118.4710	01/18/1994	155144.9	12.0	4.00	0.065	VI	9.9(16.0)
GSP	34.2930	118.3890	12/06/1994	034834.5	9.0	4.50	0.095	VII	10.0(16.1)
GSP	34.3540	118.7040	05/01/1996	194956.4	14.0	4.10	0.068	VI	10.1(16.3)
GSP	34.3650	118.7080	01/19/1994	044314.5	12.0	4.10	0.067	VI	10.2(16.5)
GSP	34.3260	118.6980	01/17/1994	233330.7	9.0	5.60	0.205	VIII	10.3(16.6)
GSB	34.3790	118.7110	01/19/1994	210928.6	14.0	5.50	0.190	VIII	10.4(16.7)
GSP	34.2310	118.4750	03/20/1994	212012.3	13.0	5.30	0.158	VIII	10.8(17.3)
GSP	34.2280	118.5730	01/17/1994	175608.2	19.0	4.60	0.094	VII	10.8(17.4)
GSP	34.2150	118.5100	01/19/1994	140914.8	17.0	4.50	0.081	VII	11.5(18.5)
GSP	34.2130	118.5370	01/17/1994	123055.4	18.0	6.70	0.362	IX	11.6(18.6)
DMG	34.4110	118.3290	02/10/1971	5 636.0	4.7	4.30	0.068	VI	11.6(18.7)
DMG	34.3390	118.3320	02/09/1971	141612.9	11.1	4.10	0.058	VI	11.6(18.7)
DMG	34.3350	118.3310	02/09/1971	155820.7	14.2	4.80	0.100	VII	11.7(18.9)
GSP	34.2180	118.6070	01/18/1994	113509.9	12.0	4.20	0.060	VI	12.1(19.4)
DMG	34.5290	118.6440	02/07/1956	21656.5	16.0	4.20	0.059	VI	12.1(19.5)
GSG	34.3040	118.7220	01/17/1994	221922.3	10.0	4.00	0.050	VI	12.2(19.6)
DMG	34.3680	118.3140	04/25/1971	1448 6.5	-2.0	4.00	0.050	VI	12.3(19.8)
DMG	34.3610	118.3060	02/09/1971	141021.5	5.0	4.70	0.083	VII	12.8(20.6)
DMG	34.3700	118.3020	02/10/1971	31212.0	0.8	4.00	0.047	VI	13.0(20.9)
GSP	34.3040	118.7370	01/19/1994	091310.9	13.0	4.10	0.050	VI	13.0(20.9)
DMG	34.5860	118.6130	02/07/1956	31638.6	2.6	4.60	0.063	VI	15.0(24.1)
DMG	34.4170	118.8330	06/01/1946	11 631.0	0.0	4.10	0.034	V	17.5(28.1)
DMG	34.5190	118.1980	08/23/1952	10 9 7.1	13.1	5.00	0.054	VI	21.1(34.0)
T-A	34.4200	118.9200	03/29/1917	8 6 0.0	0.0	4.30	0.029	V	22.4(36.1)
MGI	34.1000	118.3000	07/16/1920	2130 0.0	0.0	4.60	0.034	V	23.4(37.6)
MGI	34.1000	118.3000	07/26/1920	1215 0.0	0.0	4.00	0.021	IV	23.4(37.6)
MGI	34.1000	118.3000	07/16/1920	2022 0.0	0.0	4.60	0.034	V	23.4(37.6)
MGI	34.1000	118.3000	07/16/1920	2127 0.0	0.0	4.60	0.034	V	23.4(37.6)
GSP	34.0590	118.3870	09/09/2001	235918.0	4.0	4.20	0.024	V	23.6(38.0)
DMG	34.1000	118.8000	05/10/1911	1340 0.0	0.0	4.00	0.020	IV	24.8(39.9)
T-A	34.1700	118.1700	03/07/1888	1554 0.0	0.0	4.30	0.024	V	25.1(40.4)

MGI	34.0800	118.2600	07/16/1920	18 8 0.0	0.0	5.00	0.041	V	25.8(41.6)
DMG	34.6000	118.9000	05/18/1940	91512.0	0.0	4.00	0.018	IV	26.0(41.8)
DMG	34.6670	118.8330	01/24/1950	215659.0	0.0	4.00	0.018	IV	26.3(42.3)
MGI	34.0000	118.5000	11/19/1918	2018 0.0	0.0	5.00	0.040	V	26.3(42.4)
MGI	34.0000	118.5000	03/08/1918	1230 0.0	0.0	4.00	0.018	IV	26.3(42.4)
DMG	34.0000	118.5000	08/04/1927	1224 0.0	0.0	5.00	0.040	V	26.3(42.4)
MGI	34.0000	118.5000	06/23/1920	1220 0.0	0.0	4.00	0.018	IV	26.3(42.4)
DMG	34.0000	118.5000	06/22/1920	248 0.0	0.0	4.90	0.037	V	26.3(42.4)
DMG	34.0000	118.5000	11/08/1914	1140 0.0	0.0	4.50	0.027	V	26.3(42.4)
DMG	34.0000	118.5000	03/06/1918	1820 0.0	0.0	4.00	0.018	IV	26.3(42.4)
DMG	34.4830	118.9830	09/03/1942	14 6 1.0	0.0	4.50	0.026	V	26.8(43.1)
DMG	34.4830	118.9830	09/04/1942	63433.0	0.0	4.50	0.026	V	26.8(43.1)
DMG	34.5650	118.1130	02/28/1969	45612.4	5.3	4.30	0.022	IV	26.9(43.3)
MGI	34.1000	118.2000	04/21/1921	1538 0.0	0.0	4.00	0.017	IV	27.0(43.4)
MGI	34.1000	118.2000	01/27/1860	830 0.0	0.0	4.30	0.022	IV	27.0(43.4)
MGI	34.1000	118.2000	05/02/1916	1432 0.0	0.0	4.00	0.017	IV	27.0(43.4)
DMG	34.0000	118.4170	12/07/1938	338 0.0	0.0	4.00	0.017	IV	27.0(43.5)
MGI	34.0000	118.4000	02/22/1920	1610 0.0	0.0	4.60	0.028	V	27.3(43.9)
MGI	34.0000	118.4000	10/01/1930	040 0.0	0.0	4.60	0.028	V	27.3(43.9)

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FILE CODE	LAT. NORTH	LONG. WEST	DATE	TIME (UTC) H M Sec	DEPTH (km)	QUAKE MAG.	SITE ACC. g	SITE MM INT.	APPROX. DISTANCE mi [km]
MGI	34.0000	118.4000	01/29/1927	2324 0.0	0.0	4.00	0.017	IV	27.3(43.9)
MGI	34.0000	118.4000	02/07/1927	429 0.0	0.0	4.60	0.028	V	27.3(43.9)
PAS	34.1490	118.1350	12/03/1988	113826.4	13.3	4.90	0.035	V	27.6(44.4)
PAS	34.5410	118.9890	06/12/1984	02752.4	11.7	4.10	0.017	IV	28.4(45.7)
PAS	34.3780	119.0350	04/03/1985	4 449.8	27.9	4.00	0.016	IV	28.8(46.4)
MGI	34.0000	118.3000	06/22/1920	2035 0.0	0.0	4.00	0.015	IV	29.4(47.2)
MGI	34.0000	118.3000	09/03/1905	540 0.0	0.0	5.30	0.044	VI	29.4(47.2)
MGI	34.0000	118.3000	06/30/1920	350 0.0	0.0	4.00	0.015	IV	29.4(47.2)
DMG	33.9500	118.6320	08/31/1930	04036.0	0.0	5.20	0.039	V	30.3(48.8)
DMG	33.9830	118.3000	02/11/1940	192410.0	0.0	4.00	0.015	IV	30.4(48.9)
T-A	34.0000	118.2500	01/17/1857	1 0 0.0	0.0	4.30	0.018	IV	30.7(49.5)
T-A	34.0000	118.2500	09/23/1827	0 0 0.0	0.0	5.00	0.032	V	30.7(49.5)
T-A	34.0000	118.2500	03/21/1880	1425 0.0	0.0	4.30	0.018	IV	30.7(49.5)
T-A	34.0000	118.2500	05/02/1856	810 0.0	0.0	4.30	0.018	IV	30.7(49.5)
T-A	34.0000	118.2500	05/04/1857	6 0 0.0	0.0	4.30	0.018	IV	30.7(49.5)
T-A	34.0000	118.2500	03/26/1860	0 0 0.0	0.0	5.00	0.032	V	30.7(49.5)
T-A	34.0000	118.2500	01/10/1856	0 0 0.0	0.0	5.00	0.032	V	30.7(49.5)
GSP	34.2620	118.0020	06/28/1991	144354.5	11.0	5.40	0.044	VI	31.2(50.1)
MGI	34.1000	118.1000	07/11/1855	415 0.0	0.0	6.30	0.090	VII	31.2(50.3)
GSP	34.0300	118.1800	06/12/1989	165718.4	16.0	4.40	0.019	IV	31.4(50.5)
PAS	33.9440	118.6810	01/01/1979	231438.9	11.3	5.00	0.031	V	31.4(50.5)
GSP	34.0490	118.9150	02/19/1995	212418.1	15.0	4.30	0.018	IV	31.8(51.1)
GSP	34.0200	118.1800	06/12/1989	172225.5	16.0	4.10	0.015	IV	31.9(51.3)
PAS	33.9330	118.6690	10/17/1979	205237.3	5.5	4.20	0.016	IV	31.9(51.4)
GSP	34.2500	117.9900	06/28/1991	170055.5	9.0	4.30	0.017	IV	32.0(51.6)
MGI	34.0000	118.2000	02/13/1917	13 5 0.0	0.0	4.60	0.022	IV	32.3(52.0)
MGI	34.0000	118.2000	06/26/1917	2115 0.0	0.0	4.60	0.022	IV	32.3(52.0)
MGI	34.0000	118.2000	06/26/1917	2120 0.0	0.0	4.60	0.022	IV	32.3(52.0)
MGI	34.0000	118.2000	06/26/1917	424 0.0	0.0	4.00	0.013	III	32.3(52.0)
MGI	34.0000	118.2000	06/26/1917	2130 0.0	0.0	4.60	0.022	IV	32.3(52.0)

PAS	33.9190	118.6270	01/19/1989	65328.8	11.9	5.00	0.030	V	32.4(52.1)
PAS	34.0730	118.0980	10/04/1987	105938.2	8.2	5.30	0.038	V	32.5(52.3)
MGI	34.2000	118.0000	01/09/1921	530 0.0	0.0	4.60	0.022	IV	32.7(52.6)
PAS	34.0760	118.0900	10/01/1987	1448 3.1	11.7	4.10	0.014	IV	32.7(52.7)
PAS	34.0600	118.1000	10/01/1987	1449 5.9	11.7	4.70	0.023	IV	33.0(53.1)
DMG	33.9030	118.4310	11/29/1938	192115.8	10.0	4.00	0.013	III	33.4(53.8)
T-A	34.8300	118.7500	11/27/1852	0 0 0.0	0.0	7.00	0.137	VIII	33.5(53.9)
PAS	34.0490	118.1010	10/01/1987	144541.5	13.6	4.70	0.022	IV	33.5(53.9)
PAS	34.0540	118.9640	04/13/1982	11 212.2	16.6	4.00	0.013	III	33.5(54.0)
PAS	34.0520	118.0900	10/01/1987	151231.8	10.8	4.70	0.022	IV	33.8(54.4)
PAS	34.0610	118.0790	10/01/1987	144220.0	9.5	5.90	0.058	VI	33.9(54.5)
DMG	34.6830	119.0000	04/06/1943	223624.0	0.0	4.00	0.013	III	34.0(54.6)
PAS	34.0500	118.0870	10/01/1987	155953.5	10.4	4.00	0.013	III	34.0(54.8)
DMG	34.7170	118.9670	06/11/1935	1810 0.0	0.0	4.00	0.012	III	34.0(54.8)
DMG	34.5000	119.1170	11/17/1954	23 351.0	0.0	4.40	0.017	IV	34.5(55.5)
PAS	34.0770	118.0470	02/11/1988	152555.7	12.5	4.70	0.021	IV	34.6(55.7)
DMG	34.7000	119.0000	10/23/1916	254 0.0	0.0	5.50	0.041	V	34.7(55.8)
GSP	33.9220	118.2700	10/28/2001	162745.6	21.0	4.00	0.012	III	35.0(56.2)
DMG	34.7840	118.9020	07/27/1972	03117.4	8.0	4.40	0.017	IV	35.0(56.3)
DMG	34.0170	118.9670	04/16/1948	222624.0	0.0	4.70	0.021	IV	35.4(57.0)
DMG	34.6170	119.0830	02/26/1950	0 622.0	0.0	4.70	0.021	IV	35.5(57.1)
DMG	33.9390	118.2050	01/11/1950	214135.0	0.4	4.10	0.013	III	35.7(57.4)
MGI	34.1000	118.0000	01/27/1930	2026 0.0	0.0	4.60	0.019	IV	35.9(57.7)

EARTHQUAKE SEARCH RESULTS

FILE CODE	LAT. NORTH	LONG. WEST	DATE	TIME (UTC) H M Sec	DEPTH (km)	QUAKE MAG.	SITE ACC. g	SITE MM INT.	APPROX. DISTANCE mi [km]
DMG	34.0650	119.0350	02/21/1973	144557.3	8.0	5.90	0.053	VI	36.2(58.2)
PAS	34.0160	118.9880	10/26/1984	172043.5	13.3	4.60	0.018	IV	36.3(58.5)
DMG	33.8830	118.3170	03/11/1933	1457 0.0	0.0	4.90	0.023	IV	36.4(58.6)
DMG	33.9500	118.1330	10/25/1933	7 046.0	0.0	4.30	0.014	IV	37.4(60.1)
MGI	34.0000	119.0000	12/14/1912	0 0 0.0	0.0	5.70	0.043	VI	37.6(60.5)
DMG	34.0000	119.0000	09/24/1827	4 0 0.0	0.0	7.00	0.119	VII	37.6(60.5)
DMG	34.2000	117.9000	08/28/1889	215 0.0	0.0	5.50	0.036	V	38.0(61.1)
DMG	34.2000	117.9000	07/13/1935	105416.5	0.0	4.70	0.019	IV	38.0(61.1)
MGI	33.9000	118.2000	10/08/1927	1914 0.0	0.0	4.60	0.017	IV	38.1(61.4)
DMG	34.8670	118.8670	07/22/1952	74455.0	0.0	4.10	0.011	III	38.7(62.2)
DMG	33.9670	118.0500	01/30/1941	13446.9	0.0	4.10	0.011	III	39.6(63.7)
DMG	33.8500	118.2670	03/11/1933	629 0.0	0.0	4.40	0.014	IV	39.6(63.7)
DMG	33.8500	118.2670	03/11/1933	1425 0.0	0.0	5.00	0.023	IV	39.6(63.7)
DMG	33.8670	118.2170	06/19/1944	0 333.0	0.0	4.50	0.015	IV	39.7(63.9)
DMG	33.8670	118.2170	06/19/1944	3 6 7.0	0.0	4.40	0.014	IV	39.7(63.9)
MGI	34.0000	118.0000	05/05/1929	735 0.0	0.0	4.00	0.010	III	40.0(64.4)
MGI	34.0000	118.0000	05/05/1929	1 7 0.0	0.0	4.60	0.016	IV	40.0(64.4)
MGI	34.0000	118.0000	12/25/1903	1745 0.0	0.0	5.00	0.022	IV	40.0(64.4)
MGI	33.8000	118.5000	06/18/1915	15 5 0.0	0.0	4.00	0.010	III	40.1(64.6)
DMG	33.8670	118.2000	11/13/1933	2128 0.0	0.0	4.00	0.010	III	40.1(64.6)
MGI	34.2000	119.2000	06/16/1914	1052 0.0	0.0	4.60	0.016	IV	40.2(64.8)
DMG	33.9900	119.0580	05/29/1955	164335.4	17.4	4.10	0.011	III	40.5(65.2)
DMG	34.8670	118.9330	09/21/1941	1953 7.2	0.0	5.20	0.026	V	40.7(65.4)
PAS	34.9430	118.7430	06/10/1988	23 643.0	6.8	5.40	0.030	V	40.7(65.5)
DMG	34.8350	118.9880	11/29/1936	55445.3	10.0	4.00	0.010	III	40.8(65.6)
DMG	33.9000	118.1000	07/08/1929	1646 6.7	13.0	4.70	0.017	IV	41.3(66.4)

DMG	33.9960	117.9750	06/15/1967	458 5.5	10.0	4.10	0.010	III	41.3(	66.5)
DMG	34.4000	117.8000	02/24/1946	6 752.0	0.0	4.10	0.010	III	41.6(	66.9)
DMG	34.9000	118.9000	10/23/1916	244 0.0	0.0	6.00	0.047	VI	41.6(	66.9)
DMG	33.7830	118.4170	11/01/1940	725 3.0	0.0	4.00	0.009	III	41.8(	67.2)
DMG	33.7830	118.4170	11/02/1940	25826.0	0.0	4.00	0.009	III	41.8(	67.2)
DMG	33.7830	118.4170	10/14/1940	205111.0	0.0	4.00	0.009	III	41.8(	67.2)
DMG	33.7830	118.4170	10/12/1940	024 0.0	0.0	4.00	0.009	III	41.8(	67.2)
DMG	33.8000	118.3000	11/03/1931	16 5 0.0	0.0	4.00	0.009	III	42.2(	67.9)
MGI	33.8000	118.3000	12/31/1928	1045 0.0	0.0	4.00	0.009	III	42.2(	67.9)
DMG	33.7700	118.4800	04/24/1931	182754.8	0.0	4.40	0.013	III	42.2(	68.0)
DMG	33.7670	118.4500	10/11/1940	55712.3	0.0	4.70	0.016	IV	42.6(	68.6)
DMG	34.8430	119.0260	03/07/1939	195331.8	10.0	4.00	0.009	III	42.6(	68.6)
DMG	33.8170	118.2170	10/22/1941	65718.5	0.0	4.90	0.019	IV	42.8(	68.9)
DMG	34.9000	118.9500	08/01/1952	13 430.0	0.0	5.10	0.022	IV	43.1(	69.3)
T-A	34.9200	118.9200	05/23/1857	0 0 0.0	0.0	5.00	0.020	IV	43.4(	69.8)
T-A	34.9200	118.9200	08/29/1857	0 0 0.0	0.0	4.30	0.011	III	43.4(	69.8)
T-A	34.9200	118.9200	01/20/1857	0 0 0.0	0.0	5.00	0.020	IV	43.4(	69.8)
DMG	34.1180	119.2200	03/18/1957	185628.0	13.8	4.70	0.016	IV	43.4(	69.8)
DMG	34.8000	119.1000	09/05/1883	1230 0.0	0.0	6.00	0.044	VI	43.5(	70.0)
DMG	34.8670	119.0170	07/21/1952	2153 9.0	0.0	4.30	0.011	III	43.5(	70.1)
DMG	34.9500	118.8670	07/21/1952	121936.0	0.0	5.30	0.025	V	43.7(	70.4)
MGI	34.4000	119.3000	08/12/1925	1845 0.0	0.0	4.00	0.009	III	43.9(	70.7)
DMG	34.8850	119.0020	02/23/1939	91846.7	10.0	4.50	0.013	III	44.0(	70.8)
DMG	33.7830	118.2500	11/14/1941	84136.3	0.0	5.40	0.027	V	44.2(	71.2)
MGI	34.3000	119.3000	09/28/1926	1749 0.0	0.0	4.00	0.009	III	44.3(	71.3)
MGI	34.3000	119.3000	05/01/1904	1830 0.0	0.0	4.60	0.014	IV	44.3(	71.3)
MGI	34.3000	119.3000	05/15/1927	1120 0.0	0.0	4.00	0.009	III	44.3(	71.3)

EARTHQUAKE SEARCH RESULTS

FILE CODE	LAT. NORTH	LONG. WEST	DATE	TIME (UTC) H M Sec	DEPTH (km)	QUAKE MAG.	SITE ACC. g	SITE MM INT.	APPROX. DISTANCE mi	[km]
DMG	35.0000	118.7330	08/23/1952	6 3 3.0	0.0	4.30	0.011	III	44.3(	71.3)
DMG	35.0000	118.7330	04/29/1953	124745.0	0.0	4.70	0.015	IV	44.3(	71.3)
DMG	34.9110	118.9730	02/23/1939	84551.7	10.0	4.50	0.013	III	44.5(	71.5)
DMG	34.8830	119.0330	08/20/1952	84747.0	0.0	4.20	0.010	III	45.0(	72.4)
DMG	34.9280	118.9700	01/15/1955	1 3 6.7	9.1	4.30	0.011	III	45.3(	72.9)
DMG	33.7830	118.2000	12/27/1939	192849.0	0.0	4.70	0.015	IV	45.3(	73.0)
DMG	33.7590	118.2530	08/31/1938	31814.2	10.0	4.50	0.012	III	45.7(	73.6)
DMG	34.9320	118.9760	03/01/1963	02557.9	13.9	5.00	0.018	IV	45.7(	73.6)
DMG	34.1000	117.8000	03/31/1931	2033 0.0	0.0	4.00	0.008	III	45.9(	73.9)
DMG	34.9500	118.9500	10/16/1952	1222 7.0	0.0	4.30	0.010	III	46.0(	74.0)
DMG	35.0000	118.8330	12/01/1952	52610.0	0.0	4.40	0.011	III	46.1(	74.2)
DMG	35.0000	118.8330	07/23/1952	75319.0	0.0	5.40	0.025	V	46.1(	74.2)
DMG	35.0000	118.8330	07/23/1952	181351.0	0.0	5.20	0.021	IV	46.1(	74.2)
DMG	34.9030	119.0380	05/08/1939	248 5.3	10.0	4.50	0.012	III	46.2(	74.4)
DMG	34.9450	118.9680	03/04/1963	201042.3	8.5	4.00	0.008	III	46.3(	74.4)
GSP	34.9180	119.0200	12/24/2000	010421.9	14.0	4.40	0.011	III	46.4(	74.7)
DMG	34.9000	119.0500	07/22/1952	143018.0	0.0	4.30	0.010	III	46.5(	74.8)
DMG	34.9410	118.9870	11/15/1961	53855.5	10.7	5.00	0.018	IV	46.6(	75.0)
DMG	34.9830	118.9000	03/23/1953	17 637.0	0.0	4.00	0.008	III	46.6(	75.0)
DMG	34.9830	118.9000	07/24/1952	95032.0	0.0	4.30	0.010	III	46.6(	75.0)
PAS	33.9650	117.8860	01/01/1976	172012.9	6.2	4.20	0.009	III	46.6(	75.0)
DMG	34.9670	118.9500	11/27/1952	153641.0	0.0	4.00	0.008	II	47.0(	75.6)

DMG	34.9670	118.9500	07/30/1952	11 255.0	0.0	4.10	0.009	III	47.0(75.6)
DMG	33.7830	118.1330	01/13/1940	749 7.0	0.0	4.00	0.008	II	47.1(75.7)
DMG	33.7830	118.1330	10/02/1933	91017.6	0.0	5.40	0.024	V	47.1(75.7)
DMG	33.7830	118.1330	11/20/1933	1032 0.0	0.0	4.00	0.008	II	47.1(75.7)
GSP	34.4810	119.3530	10/23/1996	220929.4	14.0	4.20	0.009	III	47.4(76.3)
DMG	35.0630	118.4230	08/26/1952	205640.6	-0.8	4.40	0.011	III	47.5(76.4)
DMG	35.0670	118.6170	07/23/1952	235136.0	0.0	4.00	0.008	II	47.6(76.7)
DMG	33.7500	118.1830	08/04/1933	41748.0	0.0	4.00	0.008	II	47.8(77.0)
DMG	34.9500	119.0170	11/11/1952	181225.0	0.0	4.10	0.008	III	48.1(77.4)
DMG	33.7500	118.1670	05/16/1933	205855.0	0.0	4.00	0.008	II	48.2(77.6)
DMG	33.7670	118.1170	11/04/1939	2141 0.0	0.0	4.00	0.008	II	48.5(78.0)
DMG	34.9670	119.0000	09/02/1952	204556.0	0.0	4.70	0.013	III	48.5(78.1)
DMG	35.0330	118.8500	10/07/1953	145921.0	0.0	4.90	0.016	IV	48.6(78.2)
DMG	35.0830	118.5830	08/04/1952	535 0.0	0.0	4.00	0.008	II	48.6(78.2)
DMG	35.0830	118.5830	07/22/1952	81624.0	0.0	4.40	0.010	III	48.6(78.2)
GSP	34.1500	117.7200	03/01/1990	032303.0	11.0	4.70	0.013	III	48.8(78.6)
DMG	34.9330	119.0670	02/10/1954	235838.0	0.0	4.50	0.011	III	48.9(78.6)
DMG	34.9830	118.9830	05/23/1954	235243.0	0.0	5.10	0.018	IV	48.9(78.7)
PAS	33.9060	119.1660	05/23/1978	91650.8	6.0	4.00	0.007	II	49.0(78.8)
DMG	33.7500	118.1330	03/11/1933	11 4 0.0	0.0	4.60	0.012	III	49.1(79.0)
DMG	35.0670	118.7670	07/22/1952	21 211.0	0.0	4.20	0.009	III	49.3(79.3)
PAS	35.0950	118.5190	06/22/1981	45747.3	5.0	4.00	0.007	II	49.3(79.4)
DMG	34.1500	119.3500	08/22/1950	224758.0	0.0	4.20	0.009	III	49.5(79.6)
DMG	34.9220	119.1030	01/09/1963	6 4 3.8	8.7	4.00	0.007	II	49.6(79.8)
PAS	34.1360	117.7090	06/26/1988	15 458.5	7.9	4.60	0.012	III	49.8(80.1)
GSP	34.1100	117.7200	04/17/1990	223227.2	4.0	4.60	0.012	III	49.8(80.2)
DMG	35.1000	118.6170	09/26/1952	202120.0	0.0	4.00	0.007	II	49.9(80.3)
DMG	33.6630	118.4130	01/08/1967	738 5.3	17.7	4.00	0.007	II	50.0(80.4)
DMG	35.0830	118.7500	07/26/1952	15 831.0	0.0	4.40	0.010	III	50.1(80.6)
DMG	35.0830	118.7500	07/22/1952	84734.0	0.0	4.70	0.013	III	50.1(80.6)
DMG	35.0830	118.7500	07/26/1952	18 244.0	0.0	4.00	0.007	II	50.1(80.6)

EARTHQUAKE SEARCH RESULTS

FILE CODE	LAT. NORTH	LONG. WEST	DATE	TIME (UTC) H M Sec	DEPTH (km)	QUAKE MAG.	SITE ACC. g	SITE MM INT.	APPROX. DISTANCE mi [km]
DMG	34.3700	117.6500	12/08/1812	15 0 0.0	0.0	7.00	0.080	VII	50.1(80.6)
DMG	35.0330	118.9170	07/23/1952	211658.0	0.0	4.10	0.008	II	50.1(80.7)
GSP	34.1400	117.7000	02/28/1990	234336.6	5.0	5.20	0.019	IV	50.1(80.7)
GSP	34.3740	117.6490	08/20/1998	234958.4	9.0	4.40	0.010	III	50.2(80.7)
DMG	33.8000	118.0000	10/21/1913	938 0.0	0.0	4.00	0.007	II	50.2(80.8)
GSP	34.1300	117.7000	03/01/1990	003457.1	4.0	4.00	0.007	II	50.4(81.1)
DMG	35.0000	119.0000	07/21/1952	1228 0.0	0.0	4.20	0.008	III	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	1317 0.0	0.0	4.00	0.007	II	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	1222 0.0	0.0	4.90	0.015	IV	50.4(81.2)
DMG	35.0000	119.0000	07/22/1952	133143.0	0.0	4.80	0.014	III	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	1259 0.0	0.0	4.20	0.008	III	50.4(81.2)
DMG	35.0000	119.0000	07/23/1952	043 8.0	0.0	4.40	0.010	III	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	1218 0.0	0.0	4.40	0.010	III	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	12 7 0.0	0.0	4.70	0.013	III	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	1536 0.0	0.0	4.20	0.008	III	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	1553 0.0	0.0	4.50	0.011	III	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	1542 0.0	0.0	4.20	0.008	III	50.4(81.2)
DMG	35.0000	119.0000	02/16/1919	1557 0.0	0.0	5.00	0.016	IV	50.4(81.2)

DMG	35.0000	119.0000	07/21/1952	1415 0.0	0.0	4.40	0.010	III	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	1210 0.0	0.0	4.50	0.011	III	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	1212 0.0	0.0	4.60	0.012	III	50.4(81.2)
DMG	35.0000	119.0000	08/10/1952	194424.0	0.0	4.10	0.008	II	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	13 8 0.0	0.0	4.50	0.011	III	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	14 6 0.0	0.0	4.20	0.008	III	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	1311 0.0	0.0	4.10	0.008	II	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	1313 0.0	0.0	4.50	0.011	III	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	18 0 0.0	0.0	4.50	0.011	III	50.4(81.2)
DMG	35.0000	119.0000	07/22/1952	82122.0	0.0	4.10	0.008	II	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	1359 0.0	0.0	4.60	0.012	III	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	1442 0.0	0.0	4.20	0.008	III	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	1451 0.0	0.0	4.20	0.008	III	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	1638 0.0	0.0	4.50	0.011	III	50.4(81.2)
DMG	35.0000	119.0000	07/22/1952	191024.0	0.0	4.10	0.008	II	50.4(81.2)
DMG	35.0000	119.0000	07/22/1952	175236.0	0.0	4.10	0.008	II	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	1417 0.0	0.0	4.10	0.008	II	50.4(81.2)
DMG	35.0000	119.0000	03/13/1929	228 0.0	0.0	4.50	0.011	III	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	1336 0.0	0.0	4.10	0.008	II	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	1239 0.0	0.0	4.20	0.008	III	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	12 6 0.0	0.0	4.80	0.014	III	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	1240 0.0	0.0	4.90	0.015	IV	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	12 531.0	0.0	6.40	0.050	VI	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	1617 0.0	0.0	4.10	0.008	II	50.4(81.2)
DMG	35.0000	119.0000	07/25/1952	0 3 0.0	0.0	4.00	0.007	II	50.4(81.2)
DMG	35.0000	119.0000	01/25/1919	2229 0.0	0.0	4.00	0.007	II	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	132512.0	0.0	4.50	0.011	III	50.4(81.2)
DMG	35.0000	119.0000	07/21/1952	1225 0.0	0.0	4.70	0.013	III	50.4(81.2)
DMG	33.7500	118.0830	03/15/1933	432 0.0	0.0	4.10	0.008	II	50.5(81.2)
DMG	33.7500	118.0830	03/14/1933	036 0.0	0.0	4.20	0.008	III	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	911 0.0	0.0	4.40	0.010	III	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	553 0.0	0.0	4.00	0.007	II	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	1138 0.0	0.0	4.00	0.007	II	50.5(81.2)
DMG	33.7500	118.0830	03/31/1933	1049 0.0	0.0	4.10	0.008	II	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	252 0.0	0.0	4.00	0.007	II	50.5(81.2)

EARTHQUAKE SEARCH RESULTS

FILE CODE	LAT. NORTH	LONG. WEST	DATE	TIME (UTC) H M Sec	DEPTH (km)	QUAKE MAG.	SITE ACC. g	SITE MM INT.	APPROX. DISTANCE mi [km]
DMG	33.7500	118.0830	04/02/1933	8 0 0.0	0.0	4.00	0.007	II	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	2 9 0.0	0.0	5.00	0.016	IV	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	210 0.0	0.0	4.60	0.012	III	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	611 0.0	0.0	4.40	0.010	III	50.5(81.2)
DMG	33.7500	118.0830	03/14/1933	1219 0.0	0.0	4.50	0.011	III	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	230 0.0	0.0	5.10	0.017	IV	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	1129 0.0	0.0	4.00	0.007	II	50.5(81.2)
DMG	33.7500	118.0830	03/12/1933	448 0.0	0.0	4.00	0.007	II	50.5(81.2)
DMG	33.7500	118.0830	03/13/1933	1929 0.0	0.0	4.20	0.008	III	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	227 0.0	0.0	4.60	0.012	III	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	910 0.0	0.0	5.10	0.017	IV	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	2 5 0.0	0.0	4.30	0.009	III	50.5(81.2)
DMG	33.7500	118.0830	03/13/1933	343 0.0	0.0	4.10	0.008	II	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	339 0.0	0.0	4.00	0.007	II	50.5(81.2)

DMG	33.7500	118.0830	03/11/1933	521 0.0	0.0	4.40	0.010	III	50.5(	81.2)
DMG	33.7500	118.0830	03/11/1933	524 0.0	0.0	4.20	0.008	III	50.5(	81.2)
DMG	33.7500	118.0830	03/13/1933	1532 0.0	0.0	4.10	0.008	II	50.5(	81.2)
DMG	33.7500	118.0830	03/11/1933	222 0.0	0.0	4.00	0.007	II	50.5(	81.2)
DMG	33.7500	118.0830	03/30/1933	1225 0.0	0.0	4.40	0.010	III	50.5(	81.2)
DMG	33.7500	118.0830	03/11/1933	11 0 0.0	0.0	4.00	0.007	II	50.5(	81.2)
DMG	33.7500	118.0830	03/11/1933	837 0.0	0.0	4.00	0.007	II	50.5(	81.2)
DMG	33.7500	118.0830	04/01/1933	642 0.0	0.0	4.20	0.008	III	50.5(	81.2)
DMG	33.7500	118.0830	03/12/1933	034 0.0	0.0	4.00	0.007	II	50.5(	81.2)
DMG	33.7500	118.0830	03/14/1933	2242 0.0	0.0	4.10	0.008	II	50.5(	81.2)
DMG	33.7500	118.0830	03/11/1933	258 0.0	0.0	4.00	0.007	II	50.5(	81.2)
DMG	33.7500	118.0830	03/11/1933	23 5 0.0	0.0	4.20	0.008	III	50.5(	81.2)
DMG	33.7500	118.0830	03/23/1933	1831 0.0	0.0	4.10	0.008	II	50.5(	81.2)
DMG	33.7500	118.0830	03/11/1933	1045 0.0	0.0	4.00	0.007	II	50.5(	81.2)
DMG	33.7500	118.0830	03/11/1933	513 0.0	0.0	4.70	0.013	III	50.5(	81.2)
DMG	33.7500	118.0830	03/11/1933	832 0.0	0.0	4.20	0.008	III	50.5(	81.2)
DMG	33.7500	118.0830	03/11/1933	8 8 0.0	0.0	4.50	0.011	III	50.5(	81.2)
DMG	33.7500	118.0830	03/11/1933	2 4 0.0	0.0	4.90	0.015	IV	50.5(	81.2)
DMG	33.7500	118.0830	03/12/1933	2354 0.0	0.0	4.50	0.011	III	50.5(	81.2)
DMG	33.7500	118.0830	03/11/1933	336 0.0	0.0	4.00	0.007	II	50.5(	81.2)
DMG	33.7500	118.0830	03/13/1933	432 0.0	0.0	4.70	0.013	III	50.5(	81.2)
DMG	33.7500	118.0830	03/11/1933	211 0.0	0.0	4.40	0.010	III	50.5(	81.2)
DMG	33.7500	118.0830	03/13/1933	617 0.0	0.0	4.00	0.007	II	50.5(	81.2)
DMG	33.7500	118.0830	03/25/1933	1346 0.0	0.0	4.10	0.008	II	50.5(	81.2)
DMG	33.7500	118.0830	03/11/1933	216 0.0	0.0	4.80	0.014	III	50.5(	81.2)
DMG	33.7500	118.0830	03/11/1933	515 0.0	0.0	4.00	0.007	II	50.5(	81.2)
DMG	33.7500	118.0830	03/11/1933	2240 0.0	0.0	4.40	0.010	III	50.5(	81.2)
DMG	33.7500	118.0830	03/12/1933	1738 0.0	0.0	4.50	0.011	III	50.5(	81.2)
DMG	33.7500	118.0830	03/12/1933	027 0.0	0.0	4.40	0.010	III	50.5(	81.2)
DMG	33.7500	118.0830	04/02/1933	1536 0.0	0.0	4.00	0.007	II	50.5(	81.2)
DMG	33.7500	118.0830	03/12/1933	546 0.0	0.0	4.40	0.010	III	50.5(	81.2)
DMG	33.7500	118.0830	03/11/1933	257 0.0	0.0	4.20	0.008	III	50.5(	81.2)
DMG	33.7500	118.0830	03/11/1933	440 0.0	0.0	4.70	0.013	III	50.5(	81.2)
DMG	33.7500	118.0830	03/11/1933	2231 0.0	0.0	4.40	0.010	III	50.5(	81.2)
DMG	33.7500	118.0830	03/12/1933	1825 0.0	0.0	4.10	0.008	II	50.5(	81.2)
DMG	33.7500	118.0830	03/12/1933	1651 0.0	0.0	4.00	0.007	II	50.5(	81.2)
DMG	33.7500	118.0830	03/16/1933	1529 0.0	0.0	4.20	0.008	III	50.5(	81.2)
DMG	33.7500	118.0830	03/11/1933	311 0.0	0.0	4.20	0.008	III	50.5(	81.2)
DMG	33.7500	118.0830	03/12/1933	2128 0.0	0.0	4.10	0.008	II	50.5(	81.2)

EARTHQUAKE SEARCH RESULTS

FILE CODE	LAT. NORTH	LONG. WEST	DATE	TIME (UTC) H M Sec	DEPTH (km)	QUAKE MAG.	SITE ACC. g	SITE MM INT.	APPROX. DISTANCE mi [km]
DMG	33.7500	118.0830	03/11/1933	323 0.0	0.0	5.00	0.016	IV	50.5(81.2)
DMG	33.7500	118.0830	03/18/1933	2052 0.0	0.0	4.20	0.008	III	50.5(81.2)
DMG	33.7500	118.0830	03/19/1933	2123 0.0	0.0	4.20	0.008	III	50.5(81.2)
DMG	33.7500	118.0830	03/21/1933	326 0.0	0.0	4.10	0.008	II	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	926 0.0	0.0	4.10	0.008	II	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	22 0 0.0	0.0	4.40	0.010	III	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	2232 0.0	0.0	4.10	0.008	II	50.5(81.2)
DMG	33.7500	118.0830	03/13/1933	131828.0	0.0	5.30	0.020	IV	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	3 9 0.0	0.0	4.40	0.010	III	50.5(81.2)
DMG	33.7500	118.0830	03/16/1933	1530 0.0	0.0	4.10	0.008	II	50.5(81.2)

DMG	33.7500	118.0830	03/11/1933	759 0.0	0.0	4.10	0.008	II	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	1141 0.0	0.0	4.20	0.008	III	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	1547 0.0	0.0	4.00	0.007	II	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	259 0.0	0.0	4.60	0.012	III	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	1357 0.0	0.0	4.00	0.007	II	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	1956 0.0	0.0	4.20	0.008	III	50.5(81.2)
DMG	33.7500	118.0830	03/12/1933	15 2 0.0	0.0	4.20	0.008	III	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	751 0.0	0.0	4.20	0.008	III	50.5(81.2)
DMG	33.7500	118.0830	03/20/1933	1358 0.0	0.0	4.10	0.008	II	50.5(81.2)
DMG	33.7500	118.0830	03/16/1933	1456 0.0	0.0	4.00	0.007	II	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	3 5 0.0	0.0	4.20	0.008	III	50.5(81.2)
DMG	33.7500	118.0830	03/17/1933	1651 0.0	0.0	4.10	0.008	II	50.5(81.2)
DMG	33.7500	118.0830	03/23/1933	840 0.0	0.0	4.10	0.008	II	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	1147 0.0	0.0	4.40	0.010	III	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	1025 0.0	0.0	4.00	0.007	II	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	1944 0.0	0.0	4.00	0.007	II	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	347 0.0	0.0	4.10	0.008	II	50.5(81.2)
DMG	33.7500	118.0830	03/15/1933	540 0.0	0.0	4.20	0.008	III	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	635 0.0	0.0	4.20	0.008	III	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	555 0.0	0.0	4.00	0.007	II	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	439 0.0	0.0	4.90	0.015	IV	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	618 0.0	0.0	4.20	0.008	III	50.5(81.2)
DMG	33.7500	118.0830	03/12/1933	835 0.0	0.0	4.20	0.008	III	50.5(81.2)
DMG	33.7500	118.0830	03/12/1933	6 1 0.0	0.0	4.20	0.008	III	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	436 0.0	0.0	4.60	0.012	III	50.5(81.2)
DMG	33.7500	118.0830	03/12/1933	740 0.0	0.0	4.20	0.008	III	50.5(81.2)
DMG	33.7500	118.0830	03/12/1933	616 0.0	0.0	4.60	0.012	III	50.5(81.2)
DMG	33.7500	118.0830	03/15/1933	2 8 0.0	0.0	4.10	0.008	II	50.5(81.2)
DMG	33.7500	118.0830	03/11/1933	1653 0.0	0.0	4.80	0.014	III	50.5(81.2)
DMG	34.9830	119.0330	07/21/1952	235328.0	0.0	4.50	0.011	III	50.5(81.2)
DMG	35.0330	118.9330	07/22/1952	223133.0	0.0	4.70	0.013	III	50.5(81.3)
GSP	34.1400	117.6900	03/02/1990	172625.4	6.0	4.60	0.012	III	50.7(81.6)
DMG	35.0500	118.9000	09/25/1952	162136.0	0.0	4.10	0.008	II	50.8(81.7)
DMG	35.1170	118.4810	05/01/1953	64820.9	2.4	4.10	0.008	II	50.9(81.9)
DMG	35.0170	118.9830	08/17/1952	9 9 7.0	0.0	4.10	0.008	II	50.9(82.0)
DMG	35.0000	119.0170	01/12/1954	233349.0	0.0	5.90	0.033	V	50.9(82.0)
DMG	35.0000	119.0170	05/25/1953	324 1.0	0.0	4.80	0.013	III	50.9(82.0)
DMG	35.0000	119.0170	07/21/1952	115214.0	0.0	7.70	0.132	VIII	50.9(82.0)
DMG	33.7330	118.1000	03/11/1933	15 9 0.0	0.0	4.40	0.010	III	51.0(82.1)
DMG	33.7330	118.1000	03/11/1933	1447 0.0	0.0	4.40	0.010	III	51.0(82.1)
DMG	33.7330	118.1000	03/11/1933	1350 0.0	0.0	4.40	0.010	III	51.0(82.1)
GSP	35.0980	118.3060	12/31/1995	214823.1	7.0	4.00	0.007	II	51.1(82.3)
DMG	34.8410	119.2400	01/11/1958	23 847.4	10.8	4.00	0.007	II	51.4(82.7)

EARTHQUAKE SEARCH RESULTS

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FILE CODE	LAT. NORTH	LONG. WEST	DATE	TIME (UTC) H M Sec	DEPTH (km)	QUAKE MAG.	SITE ACC. g	SITE MM INT.	APPROX. DISTANCE mi [km]
DMG	35.0000	119.0330	07/21/1952	12 2 0.0	0.0	5.60	0.025	V	51.4(82.8)
DMG	35.0000	119.0330	07/21/1952	1155 0.0	0.0	4.50	0.010	III	51.4(82.8)
DMG	35.0000	119.0330	07/21/1952	1157 0.0	0.0	4.50	0.010	III	51.4(82.8)
DMG	35.0000	119.0330	07/21/1952	1159 0.0	0.0	4.50	0.010	III	51.4(82.8)
DMG	35.0000	119.0330	07/21/1952	1154 0.0	0.0	4.50	0.010	III	51.4(82.8)
DMG	35.0000	119.0330	07/21/1952	1158 0.0	0.0	4.60	0.011	III	51.4(82.8)

DMG	35.0670	118.8830	08/17/1952	21 442.0	0.0	4.30	0.009	III	51.5(82.8)
DMG	35.0670	118.8830	08/14/1952	114146.0	0.0	4.20	0.008	III	51.5(82.8)
DMG	33.6320	118.4670	01/08/1967	73730.4	11.4	4.00	0.007	II	51.8(83.4)
DMG	35.1330	118.5170	08/14/1952	72822.0	0.0	4.10	0.007	II	52.0(83.6)
DMG	35.1330	118.5170	07/28/1952	54554.0	0.0	4.20	0.008	III	52.0(83.6)
DMG	35.1330	118.5170	07/22/1952	141 2.0	0.0	4.50	0.010	III	52.0(83.6)
DMG	35.1330	118.5170	07/23/1952	152524.0	0.0	4.00	0.007	II	52.0(83.6)
DMG	35.0000	119.0500	09/12/1952	103525.0	0.0	4.50	0.010	III	52.0(83.7)
PAS	34.0060	117.7390	02/18/1989	717 4.8	3.3	4.30	0.009	III	52.0(83.7)
DMG	35.0500	118.9500	08/17/1952	614 4.0	0.0	4.00	0.007	II	52.0(83.7)
DMG	35.0500	118.9500	11/14/1952	2334 1.4	0.0	4.00	0.007	II	52.0(83.7)
DMG	34.1000	117.6830	01/09/1934	1410 0.0	0.0	4.50	0.010	III	52.0(83.7)
DMG	34.1000	117.6830	01/18/1934	214 0.0	0.0	4.00	0.007	II	52.0(83.7)
DMG	33.6330	118.4000	10/17/1934	938 0.0	0.0	4.00	0.007	II	52.1(83.9)
DMG	35.0330	119.0000	07/22/1952	101939.0	0.0	4.10	0.007	II	52.4(84.3)
DMG	35.0670	118.9330	07/23/1952	223220.0	0.0	4.10	0.007	II	52.6(84.7)
DMG	35.1330	118.7000	09/02/1952	124132.0	0.0	4.60	0.011	III	52.8(85.0)
DMG	35.0170	119.0500	08/05/1953	122059.0	0.0	4.30	0.008	III	53.0(85.2)
DMG	33.7500	118.0000	11/16/1934	2126 0.0	0.0	4.00	0.007	II	53.0(85.3)
DMG	35.0000	119.0830	11/07/1952	85535.0	0.0	4.60	0.011	III	53.1(85.4)
PAS	35.0460	119.0010	06/05/1975	144645.3	9.0	4.10	0.007	II	53.2(85.6)
DMG	35.0450	119.0040	03/23/1956	212327.1	12.1	4.30	0.008	III	53.2(85.6)
DMG	34.3000	117.6000	07/30/1894	512 0.0	0.0	6.00	0.033	V	53.3(85.7)
DMG	34.1000	119.4000	05/19/1893	035 0.0	0.0	5.50	0.022	IV	53.3(85.8)
DMG	35.1500	118.6330	01/27/1954	141948.0	0.0	5.00	0.015	IV	53.4(86.0)
GSP	33.9550	117.7460	12/14/2001	120135.5	13.0	4.00	0.007	II	53.5(86.1)
USG	34.4180	119.4680	09/07/1984	11 345.2	9.5	4.00	0.007	II	53.5(86.2)
GSP	33.9170	117.7760	09/03/2002	070851.9	12.0	4.80	0.012	III	53.6(86.3)
DMG	35.1330	118.7670	07/21/1952	194122.0	0.0	5.50	0.022	IV	53.7(86.4)
DMG	35.1330	118.7670	07/25/1952	143442.0	0.0	4.40	0.009	III	53.7(86.4)
PAS	35.0000	119.1030	05/13/1975	02135.6	19.1	4.50	0.010	III	53.8(86.5)
DMG	35.1500	118.6830	08/13/1952	173925.0	0.0	4.70	0.011	III	53.8(86.6)
MGI	33.8000	117.9000	05/22/1902	740 0.0	0.0	4.30	0.008	III	53.9(86.7)
DMG	35.0330	119.0500	08/18/1952	44010.0	0.0	4.70	0.011	III	53.9(86.7)
DMG	35.0330	119.0500	08/07/1952	163151.0	0.0	4.90	0.013	III	53.9(86.7)
DMG	35.0330	119.0500	07/27/1952	71611.0	0.0	4.10	0.007	II	53.9(86.7)
DMG	33.7000	118.0670	03/11/1933	85457.0	0.0	5.10	0.016	IV	53.9(86.8)
DMG	33.7000	118.0670	02/08/1940	165617.0	0.0	4.00	0.006	II	53.9(86.8)
DMG	33.7000	118.0670	03/11/1933	51022.0	0.0	5.10	0.016	IV	53.9(86.8)
DMG	33.7000	118.0670	07/20/1940	4 113.0	0.0	4.00	0.006	II	53.9(86.8)
DMG	35.0670	118.9830	08/04/1952	194750.0	0.0	4.00	0.006	II	53.9(86.8)
MGI	34.0000	117.7000	12/03/1929	9 5 0.0	0.0	4.00	0.006	II	54.2(87.2)
DMG	34.3490	119.4920	07/14/1958	52555.3	16.0	4.70	0.011	III	54.9(88.4)
DMG	34.3040	117.5700	05/05/1969	16 2 9.6	8.8	4.40	0.009	III	54.9(88.4)
DMG	33.6330	118.2000	11/01/1940	20 046.0	0.0	4.00	0.006	II	54.9(88.4)
DMG	33.6300	118.2000	09/13/1929	132338.2	0.0	4.00	0.006	II	55.1(88.7)
DMG	35.0670	119.0330	07/27/1952	113438.0	0.0	4.10	0.007	II	55.3(89.1)

EARTHQUAKE SEARCH RESULTS

FILE CODE	LAT. NORTH	LONG. WEST	DATE	TIME (UTC) H M Sec	DEPTH (km)	QUAKE MAG.	SITE ACC. g	SITE MM INT.	APPROX. DISTANCE mi [km]
DMG	35.0670	119.0330	07/23/1952	175329.0	0.0	4.10	0.007	II	55.3(89.1)
DMG	33.6830	118.0500	03/11/1933	658 3.0	0.0	5.50	0.021	IV	55.4(89.2)

DMG	33.6830	118.0500	03/11/1933	1250 0.0	0.0	4.40	0.009	III	55.4	(89.2)
GSP	33.9510	117.7090	01/05/1998	181406.5	11.0	4.30	0.008	III	55.5	(89.2)
DMG	35.0330	119.1000	01/12/1954	234037.0	0.0	4.10	0.007	II	55.5	(89.3)
DMG	35.0330	119.1000	09/02/1953	152756.0	0.0	4.00	0.006	II	55.5	(89.3)
DMG	35.0330	119.1000	02/07/1954	0 953.0	0.0	4.40	0.009	III	55.5	(89.3)
DMG	35.0330	119.1000	01/13/1954	14531.0	0.0	4.40	0.009	III	55.5	(89.3)
DMG	35.1000	118.9670	08/25/1952	62026.0	0.0	4.70	0.011	III	55.5	(89.4)
DMG	35.1830	118.6000	07/26/1952	2241 3.0	0.0	4.60	0.010	III	55.5	(89.4)
DMG	35.1830	118.6000	07/26/1952	63850.0	0.0	4.00	0.006	II	55.5	(89.4)
DMG	35.1830	118.6000	07/29/1952	154950.0	0.0	4.90	0.013	III	55.5	(89.4)
DMG	34.1830	117.5830	10/03/1948	24628.0	0.0	4.00	0.006	II	55.7	(89.6)
DMG	35.0660	119.0490	01/24/1974	5 2 0.8	6.4	4.30	0.008	II	55.8	(89.7)
DMG	35.1830	118.6500	07/21/1952	151358.0	0.0	5.10	0.015	IV	55.8	(89.8)
DMG	34.5000	119.5000	06/29/1926	2321 0.0	0.0	5.50	0.021	IV	55.9	(89.9)
DMG	34.5000	119.5000	08/05/1930	1125 0.0	0.0	5.00	0.014	IV	55.9	(89.9)
DMG	34.5000	119.5000	12/05/1920	1158 0.0	0.0	4.50	0.009	III	55.9	(89.9)
PAS	35.0180	119.1410	11/10/1981	223435.5	3.1	4.50	0.009	III	56.1	(90.2)
DMG	34.2500	119.5000	04/21/1917	659 0.0	0.0	4.00	0.006	II	56.1	(90.3)
DMG	34.2500	119.5000	04/13/1917	359 0.0	0.0	4.50	0.009	III	56.1	(90.3)
DMG	34.2810	117.5520	09/13/1970	44748.6	8.0	4.40	0.008	III	56.1	(90.3)
DMG	35.1940	118.4650	07/22/1952	19 858.2	3.7	4.30	0.008	II	56.3	(90.6)
DMG	35.0670	119.0670	02/24/1954	223022.0	0.0	4.50	0.009	III	56.4	(90.7)
DMG	35.1000	119.0000	07/24/1952	311 7.0	0.0	4.10	0.007	II	56.4	(90.8)
DMG	35.1000	119.0000	07/22/1952	14 511.0	0.0	4.30	0.008	II	56.4	(90.8)
DMG	35.1990	118.5310	09/01/1961	165148.9	4.5	4.00	0.006	II	56.5	(90.9)
PAS	35.0350	119.1370	06/16/1978	42131.6	1.8	4.30	0.008	II	56.8	(91.5)
DMG	34.2670	119.5170	04/12/1944	153310.0	0.0	4.00	0.006	II	56.9	(91.5)
DMG	35.2000	118.6330	07/22/1952	321 5.0	0.0	4.40	0.008	III	56.9	(91.5)
DMG	34.2700	117.5400	09/12/1970	143053.0	8.0	5.40	0.019	IV	56.9	(91.6)
PAS	35.0120	119.1790	11/10/1981	2237 5.0	9.4	4.20	0.007	II	57.1	(91.9)
DMG	33.6710	118.0120	10/20/1961	223534.2	5.6	4.10	0.006	II	57.2	(92.1)
DMG	33.6800	117.9930	11/20/1961	85334.7	4.4	4.00	0.006	II	57.3	(92.2)
DMG	33.8540	117.7520	10/04/1961	22131.6	4.3	4.10	0.006	II	57.4	(92.4)
DMG	35.0500	119.1330	08/06/1953	1120 4.0	0.0	4.40	0.008	III	57.5	(92.6)
DMG	35.0500	119.1330	05/23/1953	75255.0	0.0	4.20	0.007	II	57.5	(92.6)
DMG	34.1830	117.5480	09/01/1937	163533.5	10.0	4.50	0.009	III	57.6	(92.7)
DMG	33.6170	118.1170	01/20/1934	2117 0.0	0.0	4.50	0.009	III	57.8	(92.9)
MGI	33.8000	117.8000	05/19/1917	719 0.0	0.0	4.00	0.006	II	57.8	(93.1)
MGI	33.8000	117.8000	05/20/1917	945 0.0	0.0	4.00	0.006	II	57.8	(93.1)
MGI	33.8000	117.8000	05/19/1917	635 0.0	0.0	4.00	0.006	II	57.8	(93.1)
MGI	33.8000	117.8000	11/04/1926	2238 0.0	0.0	4.60	0.010	III	57.8	(93.1)
MGI	33.8000	117.8000	11/07/1926	1948 0.0	0.0	4.60	0.010	III	57.8	(93.1)
MGI	33.8000	117.8000	11/10/1926	1723 0.0	0.0	4.60	0.010	III	57.8	(93.1)
MGI	33.8000	117.8000	11/09/1926	1535 0.0	0.0	4.60	0.010	III	57.8	(93.1)
DMG	34.2110	117.5300	09/01/1937	1348 8.2	10.0	4.50	0.009	III	58.2	(93.6)
PAS	34.2110	117.5300	10/19/1979	122237.8	4.9	4.10	0.006	II	58.2	(93.6)
DMG	34.2670	117.5180	09/12/1970	141011.2	8.0	4.10	0.006	II	58.2	(93.6)
DMG	35.2170	118.6670	09/14/1952	204324.0	0.0	4.10	0.006	II	58.3	(93.8)
DMG	35.2290	118.5130	06/28/1957	1132 0.8	1.6	4.10	0.006	II	58.6	(94.3)
DMG	33.6650	117.9790	10/20/1961	214240.7	7.2	4.00	0.006	II	58.6	(94.3)
DMG	35.0500	119.1670	12/14/1950	135623.0	0.0	4.40	0.008	III	58.7	(94.5)

EARTHQUAKE SEARCH RESULTS

FILE	LAT.	LONG.	DATE	TIME (UTC)	DEPTH	QUAKE	SITE ACC.	SITE MM	APPROX. DISTANCE

CODE	NORTH	WEST		H M Sec	(km)	MAG.	g	INT.	mi [km]
DMG	34.1670	117.5330	03/01/1948	81213.0	0.0	4.70	0.010	III	58.7(94.5)
DMG	33.7670	117.8170	08/22/1936	521 0.0	0.0	4.00	0.006	II	58.8(94.6)
DMG	35.1000	119.0830	07/24/1946	019 8.0	0.0	4.00	0.006	II	58.8(94.6)
DMG	35.1000	119.0830	12/06/1934	743 0.0	0.0	4.00	0.006	II	58.8(94.6)
DMG	33.6540	117.9940	10/20/1961	194950.5	4.6	4.30	0.007	II	58.8(94.6)
DMG	33.5430	118.3400	09/14/1963	35116.2	2.2	4.20	0.007	II	58.8(94.7)
DMG	35.2330	118.5330	07/30/1952	144650.0	0.0	4.10	0.006	II	58.9(94.7)
DMG	35.2330	118.5330	07/24/1952	1735 6.0	0.0	4.20	0.007	II	58.9(94.7)
DMG	35.2330	118.5330	07/21/1952	174244.0	0.0	5.10	0.014	IV	58.9(94.7)
DMG	35.2330	118.5330	07/22/1952	15 314.0	0.0	4.20	0.007	II	58.9(94.7)
DMG	35.2330	118.5330	03/17/1953	161517.0	0.0	4.00	0.006	II	58.9(94.7)
DMG	35.2330	118.5330	07/29/1952	173643.0	0.0	4.40	0.008	III	58.9(94.7)
DMG	33.6590	117.9810	10/20/1961	20 714.5	6.1	4.00	0.006	II	58.9(94.7)
DMG	34.3000	117.5000	07/22/1899	2032 0.0	0.0	6.50	0.043	VI	58.9(94.8)
PAS	33.6300	119.0200	10/23/1981	172816.9	12.0	4.60	0.009	III	58.9(94.9)
DMG	35.2330	118.6000	07/22/1952	91025.0	0.0	4.50	0.009	III	59.0(94.9)
DMG	35.2330	118.6000	01/10/1953	221738.0	0.0	4.00	0.006	II	59.0(94.9)
DMG	35.2350	118.5480	03/03/1973	181449.5	8.0	4.00	0.006	II	59.0(94.9)
MGI	33.7000	117.9000	07/08/1902	945 0.0	0.0	4.00	0.006	II	59.2(95.3)
PAS	33.6710	119.1110	09/04/1981	155050.3	5.0	5.30	0.016	IV	59.2(95.3)
DMG	35.2390	118.5180	07/21/1952	2021 5.1	-2.0	4.20	0.007	II	59.3(95.4)
DMG	35.2410	118.5600	07/21/1952	1912 7.4	5.8	4.30	0.007	II	59.4(95.6)
PAS	33.6370	119.0560	10/23/1981	191552.5	6.3	4.60	0.009	III	59.5(95.8)
DMG	34.2670	119.5670	06/29/1968	191357.0	10.0	4.40	0.008	II	59.7(96.1)
DMG	33.6170	118.0330	05/21/1938	944 0.0	0.0	4.00	0.006	II	59.9(96.4)
DMG	35.2170	118.8170	12/15/1953	124436.0	0.0	4.60	0.009	III	60.0(96.6)
DMG	35.2170	118.8170	07/23/1952	1317 5.0	0.0	5.70	0.022	IV	60.0(96.6)
DMG	34.2000	117.5000	06/14/1892	1325 0.0	0.0	4.90	0.012	III	60.0(96.6)
DMG	34.3670	119.5830	07/01/1941	75054.8	0.0	5.90	0.026	V	60.1(96.6)
DMG	35.2500	118.4830	07/23/1952	93842.0	0.0	4.20	0.007	II	60.1(96.7)
DMG	35.2500	118.4830	07/23/1952	1330 4.0	0.0	4.40	0.008	II	60.1(96.7)
DMG	34.1270	117.5210	12/27/1938	10 928.6	10.0	4.00	0.006	II	60.1(96.8)
DMG	34.3330	119.5830	10/02/1938	1845 0.0	0.0	4.00	0.006	II	60.2(96.8)
DMG	34.3330	119.5830	07/02/1941	2219 0.0	0.0	4.00	0.006	II	60.2(96.8)
DMG	34.3330	119.5830	07/01/1941	821 0.0	0.0	4.00	0.006	II	60.2(96.8)
DMG	34.3330	119.5830	07/03/1941	1926 0.0	0.0	4.00	0.006	II	60.2(96.8)
DMG	34.3330	119.5830	07/01/1941	830 0.0	0.0	4.00	0.006	II	60.2(96.8)
DMG	34.3330	119.5830	09/25/1941	51256.0	0.0	4.00	0.006	II	60.2(96.8)
DMG	34.3330	119.5830	09/08/1941	31423.0	0.0	4.00	0.006	II	60.2(96.8)
DMG	34.3330	119.5830	07/12/1941	1618 0.0	0.0	4.50	0.008	III	60.2(96.8)
DMG	34.3330	119.5830	07/01/1941	1820 0.0	0.0	4.00	0.006	II	60.2(96.8)
DMG	34.3330	119.5830	07/01/1941	1025 0.0	0.0	4.00	0.006	II	60.2(96.8)
DMG	34.3330	119.5830	09/14/1941	14518.0	0.0	4.00	0.006	II	60.2(96.8)
DMG	34.3330	119.5830	07/01/1941	2354 0.0	0.0	4.50	0.008	III	60.2(96.8)
DMG	34.3330	119.5830	07/01/1941	819 0.0	0.0	4.00	0.006	II	60.2(96.8)
DMG	34.3330	119.5830	11/18/1941	18 810.0	0.0	4.00	0.006	II	60.2(96.8)
DMG	34.3330	119.5830	11/21/1941	1656 3.0	0.0	4.00	0.006	II	60.2(96.8)
DMG	34.3330	119.5830	09/08/1941	31245.0	0.0	4.50	0.008	III	60.2(96.8)
DMG	34.3330	119.5830	07/01/1941	9 5 0.0	0.0	4.00	0.006	II	60.2(96.8)
DMG	34.3330	119.5830	09/15/1941	137 2.0	0.0	4.00	0.006	II	60.2(96.8)
DMG	34.3330	119.5830	07/01/1941	945 0.0	0.0	4.00	0.006	II	60.2(96.8)
DMG	34.3330	119.5830	07/01/1941	858 0.0	0.0	4.00	0.006	II	60.2(96.8)
DMG	34.3330	119.5830	07/01/1941	848 0.0	0.0	4.00	0.006	II	60.2(96.8)

EARTHQUAKE SEARCH RESULTS

FILE CODE	LAT. NORTH	LONG. WEST	DATE	TIME (UTC) H M Sec	DEPTH (km)	QUAKE MAG.	SITE ACC. g	SITE MM INT.	APPROX. DISTANCE mi [km]
DMG	34.1400	117.5150	01/01/1965	8 418.0	5.9	4.40	0.008	II	60.2(96.9)
DMG	33.6170	118.0170	10/02/1933	1326 1.0	0.0	4.00	0.006	II	60.3(97.1)
DMG	33.6170	118.0170	03/15/1933	111332.0	0.0	4.90	0.011	III	60.3(97.1)
DMG	33.6170	118.0170	03/14/1933	19 150.0	0.0	5.10	0.013	III	60.3(97.1)
DMG	33.9860	119.4750	08/06/1973	232917.0	16.9	5.00	0.012	III	60.5(97.4)
DMG	35.1500	119.0500	11/11/1952	1722 8.0	0.0	4.20	0.006	II	60.8(97.8)
PAS	33.5380	118.2070	05/25/1982	134430.3	13.7	4.10	0.006	II	61.0(98.2)
DMG	35.0500	119.2330	08/19/1952	191226.0	0.0	4.50	0.008	III	61.1(98.3)
DMG	34.2450	119.5880	06/29/1968	203633.6	1.8	4.00	0.005	II	61.1(98.3)
GSP	33.8060	117.7150	03/07/2000	002028.2	11.0	4.00	0.005	II	61.2(98.4)
DMG	33.6000	118.0170	12/25/1935	1715 0.0	0.0	4.50	0.008	III	61.4(98.7)
DMG	34.0000	119.5000	03/19/1905	440 0.0	0.0	4.00	0.005	II	61.4(98.7)
MGI	34.0000	119.5000	05/03/1926	1353 0.0	0.0	4.30	0.007	II	61.4(98.7)
DMG	34.0000	119.5000	02/18/1926	1818 0.0	0.0	5.00	0.012	III	61.4(98.7)
DMG	35.2670	118.4500	07/21/1952	191619.0	0.0	4.30	0.007	II	61.4(98.7)
DMG	34.2170	117.4670	03/25/1941	234341.0	0.0	4.00	0.005	II	61.6(99.2)
DMG	33.9500	117.5830	04/11/1941	12024.0	0.0	4.00	0.005	II	61.7(99.3)
DMG	33.6170	117.9670	03/11/1933	154 7.8	0.0	6.30	0.034	V	61.8(99.4)
DMG	33.6000	118.0000	03/11/1933	231 0.0	0.0	4.40	0.007	II	61.8(99.5)
DMG	33.6000	118.0000	03/11/1933	217 0.0	0.0	4.50	0.008	III	61.8(99.5)

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-END OF SEARCH- 656 EARTHQUAKES FOUND WITHIN THE SPECIFIED SEARCH AREA.

TIME PERIOD OF SEARCH: 1800 TO 2006

LENGTH OF SEARCH TIME: 207 years

THE EARTHQUAKE CLOSEST TO THE SITE IS ABOUT 1.8 MILES (2.9 km) AWAY.

LARGEST EARTHQUAKE MAGNITUDE FOUND IN THE SEARCH RADIUS: 7.7

LARGEST EARTHQUAKE SITE ACCELERATION FROM THIS SEARCH: 0.466 g

COEFFICIENTS FOR GUTENBERG & RICHTER RECURRENCE RELATION:
a-value= 3.885
b-value= 0.849
beta-value= 1.955

TABLE OF MAGNITUDES AND EXCEEDANCES:

Earthquake Magnitude	Number of Times Exceeded	Cumulative No. / Year
4.0	656	3.16908
4.5	248	1.19807
5.0	86	0.41546
5.5	31	0.14976
6.0	14	0.06763
6.5	6	0.02899
7.0	4	0.01932
7.5	1	0.00483